Attachment F

Draft Stormwater Management Report – Proposed Onshore Substation

Stormwater Management Report (Draft for DEIR Submittal)

Project:

275/345KV Substation for New England Wind 2 Connector Project Land at Clay Hill, Barnstable, MA 02050 Parcels 195037 and 195005 - 195007

Applicant/Developer:

Avangrid Offshore Wind, LLC 125 High Street Boston, MA 02110

Submitted to MEPA Office 100 Cambridge Street Suite 900 Boston, MA 02114

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Project No. 198804104

February 10, 2023



TABLE OF CONTENTS

Section 1 Stormwater Management Design and Compliance

- 1.1 Project Background and Description
- 1.2 Project Scope of Work
- 1.3 Analysis Overview
- 1.4 Existing Conditions
- 1.5 Proposed Conditions
- 1.6 Comments on Nitrogen Control Issues
- 1.7 Analysis Summary
- Section 2 Analysis for 2-year, 10-year and 100-year Storms Existing Conditions HydroCAD Analysis Proposed Conditions HydroCAD Analysis
- Section 3 Erosion & Sediment Control Plan
- Section 4 Operations and Maintenance Plan for Proposed Stormwater BMPs (to be submitted during final design)
- Section 5 Massachusetts Checklist for Stormwater Report
- Section 6 Nitrogen Loading Calculation

APPENDED DRAWING SHEETS

- SHEET 1 COVER SHEET
- SHEET 2 GENERAL NOTES
- SHEET 3 EXISTING CONDITIONS
- SHEET 4 EXISTING SUBCATCHMENT AREAS
- SHEET 5 PROPOSED EQUIPMENT LAYOUT
- SHEETS 6-7 ACCESS ROAD LAYOUT & 345KV DUCT BANK LAYOUT
- SHEET 8 PROPOSED GRADING AND DRAINAGE
- SHEET 9 PROPOSED SUBCATCHMENT AREAS
- SHEET 10 TYPICAL DETAIL SHEET



1.0 STORMWATER MANAGEMENT DESIGN AND COMPLIANCE

1.1 Project Background and Description

The proposed project site is comprised of four adjacent properties in Barnstable, Massachusetts. Property areas, from west to east, are 5.3 acres, 7.2 acres, 7.5 acres and 3.9 acres. All 4 properties are currently entirely forested, with the exception of a small 'panhandle' spur in the north of the center property that is partially occupied by a clearcut electrical easement. The proposed substation will occupy a portion of the areas of the properties, primarily in the south and center. An existing access road, leading from Oak Street to the fire tower at Clay Hill, passes the southern boundary of the three easternmost properties and will be connected to the proposed substation.

The project site is bounded by Town of Barnstable Conservation Commission land to the north, which the electrical easement passes through. The eastern and western boundaries of the site are land owned by the Town of Barnstable Conservation Commission. US Route 6 bounds the westernmost two properties to the south, while the fire tower and access road at Clay Hill is situated between Route 6 and the easternmost three properties.

There are no wetlands within 100-feet of the substation site, as it is located in the mid-Cape area, on the side slope of a glacial moraine with well-drained granular soils. There are also no perennial streams located within 200-feet of the Site. According to the Town of Barnstable Comprehensive Wastewater Management Plan dated November 2020, groundwater elevations at the site are 35-40ft, giving depths to groundwater across the site ranging from approximately 160ft in the south to 60ft in the north.

The approach to stormwater management for this project is to balance the needs of the project while preserving the integrity of groundwater and minimizing impacts to the adjacent lands. To the extent feasible, environmentally sensitive design and low impact development (LID) measures will be incorporated into the planning and design of this project. The proposed stormwater management system incorporates Best Management Practices (BMPs), as described in the Department of Environmental Protection Stormwater Management Policy Handbook, and as recommended in the Town of Barnstable's site plan criteria. These BMPs will primarily function to minimize potential adverse water quality impacts to groundwater and to downgradient receptors. The BMPs proposed will also maintain or reduce peak stormwater discharge rates released off-site, ensuring no erosive conditions will be generated; and storage/infiltration basins are proposed to ensure that post-development runoff volumes will not exceed predevelopment runoff volumes. To ensure the highest level of groundwater protection, it is proposed to provide surplus containment beneath certain substation equipment containing large quantities of dielectric fluids.

1.2 Project Scope of Work

The proposed project involves the following scope of work:

- Construction of retaining walls and associated grading such that grades within the substation, excluding access roads and ramps, are limited to no greater than 2% to facilitate operations and maintenance. A key objective of the proposed site grading will be to balance earth cuts and fills to minimize movement of soils to and from the site.
- Construction of an approx. 20' wide gravel road around the perimeter of the substation to allow for access to all proposed electrical equipment. The southeastern corner of this perimeter road will include a connection to the Clay Hill fire tower access road to the south the site, which then connects to Oak St further east. The remaining areas within the substation yard will be surfaced with crushed stone.
- A perimeter fence will be installed around the perimeter of the substation and at both entrances/exits.
- Construction of all electrical equipment and buildings as shown on the plan. Some of the equipment may feature barrier walls to help mitigate sound impacts.
- Construction of containment structures for equipment that will contain dielectric fluids (6 Transformers, 2 Station Service Transformers and 6 Shunt Reactors). Such equipment will be placed within containment structures sized to contain 110% of the dielectric fluid volume of the equipment contained, plus an additional 30-inches of vertical storage to account for rainfall during the Probable Maximum Precipitation event. Twelve (12) containment structures are shown on the proposed plans.
- A closed drainage system for conveying clean stormwater from containment areas to the infiltration basin through a final oil/water separator structure.
- A stormwater management system is provided, as further described in this report, to manage stormwater runoff from the new building, paved access ways, and crushed stone surface of the electrical equipment yard.
- A 12' wide gravel access route is to be built, connecting the substation site to stormwater management features elsewhere within the property for maintenance access purposes.

As the substation design and Site Plan are refined in the future, the project Stormwater Management Plan described in this report will be adjusted accordingly, to reflect any hydraulic or hydrologic changes or BMP changes that might result from Site Plan revision.

1.3 Analysis Overview

A stormwater drainage analysis for this project has been prepared and is presented in the sections which follow. This analysis evaluates the capacity of the proposed drainage systems, and documents compliance with the Stormwater Management Standards of the Massachusetts Department of Environmental Protection (DEP) and the Town of Barnstable's Site Plan Criteria.

The drainage analysis includes calculated estimates of the runoff volume and peak storm flow rates for each individual drainage area at the Site. HydroCAD, a software program, developed by Applied Microcomputer Systems, was utilized in the preparation of the stormwater runoff model. HydroCAD is based on the Soil Conservation Service (SCS) "Technical Release 20 – Urban Hydrology for Small Watersheds" and is a generally accepted industry standard methodology.

The Resilient Massachusetts Action Team (RMAT) Climate resilience Design Standards and Guidelines were used to determine the appropriate design rainfall event and corresponding precipitation depth. Epsilon Associates, Inc., working on behalf of Avangrid Offshore Wind, worked through the RMAT Climate Resilience Design Standards Tool to determine these design inputs. Based on factors including the criticality and design life of the proposed development, the tool output showed that the development should utilize 'Tier 3' calculation methodologies and a return period of 50 years. The tool output also included the design 50-year 24-hr precipitation depth of 8.2", calculated using tier 3 methodologies, removing the need to perform this calculation separately. Utilizing tools within HydroCAD software, an SCS Type III storm distribution curve was then used with the 24-hr storm depth to estimate peak intensities at a sub-hourly level. Rainfall depths that were utilized for design are noted below (2-year, 10-year and 100-year rain event data is the 'Extreme Precipitation Estimates' from the Northeast Regional Climate Center):

24-Hour Storm Event	Rainfall (inches)
2-year	3.3
10-year	4.8
50-year (RMAT)	8.2
100-year	8.4

Time of concentration (T_c) values and runoff curve numbers (CN) were developed for each of the calculated drainage areas based upon prevalent topographic patterns, ground cover conditions, and SCS Hydrologic Soil Group classifications. A minimum T_c of 5 minutes was used for sub-catchments with tributary areas having a calculated T_c of less than 5 minutes.

In addition to urban cover, onsite soils are comprised of two categories: Barnstable-Plymouth-Nantucket complex, rolling, very bouldery (Hydrological Soil Group A); and Plymouth-Barnstable complex, hilly, very bouldery (Hydrological Soil Group A) according to the online Web Soil Survey of the USADA Natural Resources Conservation Service (NRCS). No test pits have been excavated on Site, but soils will be thoroughly tested as part of final design and permitting. Based on the NRCS soils data, a Rawls rate of 2.41 in/hr was assumed as the site soils infiltration rate. This infiltration rate will be updated accordingly once on-site soil evaluations have taken place.

1.4 Existing Conditions

The total area of all four properties is 23.9 acres, of which 9.91 acres make up the area of the proposed substation. As described in Section 1.1, the existing site is currently entirely forested, with the exception of a small 'panhandle' spur in the north of the easternmost property that is partially occupied by a clear-cut electrical easement.

An Existing Conditions Tributary Area Plan is attached (Section 3). SCS Method¹ CN and time of concentration values were calculated to determine the peak runoff rates and volumes for each existing sub-catchment area.

The highest elevation at the Site is approximately 195' above mean sea level (msl) in the south of the site, while the lowest elevation is approximately 83' in the north-center of the site. Generally, much of the site topography slopes from the south to the low point in the north, although smaller sub-catchments are present along the boundaries of the site.

1.5 Proposed Conditions

In the post-development condition, all drainage from the proposed substation will be directed toward the localized depression in the north-eastern corner of the site, with a berm to be added to this area in order to prevent overflow of excess stormwater into the adjacent property to the east.

Inflow to the infiltration systems has been separated into sub-catchments. The catchment locations and composition of areas (i.e., roof, crushed rock surface, gravel roads, grass, woods, etc.) are shown on the Proposed Subcatchment Areas plan (Sheet 7). Post-development stormwater will substantially infiltrate on-site because the substation yard surface will be predominantly permeable (e.g. proposed crushed stone yard), with well-drained soils underneath as described in Section 1.4. However, during extreme rainfall events, rainfall and runoff from impermeable surfaces on the site may briefly exceed the infiltration capacity of the underlying soil beneath the crushed stone surfacing and will instead flow into the site drainage system.

As the site is currently forested, the impermeable area of the site will increase by approximately 3.1 acres. With the exception of the gravel maintenance access route, all excess runoff will be infiltrated or attenuated within the localized depression such that there will be no additional discharge from during the analyzed 24-hour rainfall events. Any substation equipment that contains dielectric fluids will be located within appropriately sized spill containment areas. Some of the proposed substation occupies areas of the site that previously discharged runoff off-site during extreme rainfall events, which means that a portion of this runoff will now be managed on-site.

The proposed stormwater management system incorporates Low Impact Development (LID) strategies, which are designed to capture, treat, and recharge stormwater runoff. These measures provide a treatment train to improve the quality of stormwater runoff,

¹ Soil Conservation Service hydrologic method TR-55 was used to develop the Curve Number (CN) and Time of Concentration (Tc) values used for hydrologic analysis of pre-and post-development stormwater runoff values.

reduce the quantity of stormwater runoff, and provide infiltration and recharge to groundwater. These are considered Best Management Practices (BMPs) by the Massachusetts Department of Environmental Protection. A Summary of the LID measures to be incorporated is provided below:

- Perforated under-drains will be installed throughout the site, which will collect stormwater that has percolated through the crushed rock surfaces and direct it towards the attenuation and infiltration structures. Stormwater that percolates through the crushed rock will receive a degree of filtration that removes some suspended solid pollutants.
- A hydrodynamic vortex separator device will be installed to treat all runoff from the perforated under-drains. See Section 1.5.1 for additional information.
- Some stormwater will instead flow overland into a grassed swale around the perimeter of the site, which also provides opportunity for settlement and filtration of pollutants. Outflows from the swale will then flow into a sediment forebay for additional treatment.
- Both the vortex separator device and the sediment forebay will then flow through a rip-rap lined channel down a steep slope the infiltration basin.
- The infiltration basin also collects and infiltrates runoff from undeveloped areas of the property.
- A berm/dam structure will be installed within the existing localized depression area, at the edge of the proposed infiltration basin, such that no outflow from the proposed substation will leave the site during storms up to and including the 50-year 24-hr design rainfall event.

A more detailed description of the proposed stormwater BMP features follows:

1.5.1 Structural Best Management Practices (BMPs)

As outlined previously, the detention basin will receive pretreated stormwater runoff from the aforementioned conveyance and treatment BMPs. Stormwater will be pretreated to remove at least 44% of total suspended solids (TSS) before being released into the infiltration basin in accordance with the Massachusetts Stormwater Policy as applicable to areas with a rapid infiltration rate.

Infiltration Chambers and Vortex Separator Device

A hydrodynamic vortex separator device, such as the 'Downstream Defender' by Hydro International, is proposed upstream of the infiltration basin to achieve the required TSS removal for stormwater collected by perforated drains in the interior of the substation. This product has not been certified in Massachusetts, but testing from the New Jersey Department of Environmental Protection demonstrated that it achieves TSS removal of 50% when designed, operated and maintained appropriately². The device

² https://www.hydro-int.com/sites/default/files/njdep_certification_letter_njcat_report.pdf

sizing/configuration will be confirmed with the manufacturer during the detailed design phase such that it performs optimally during the 1" design storm.

Swale and Sediment Forebay

Vegetated swales will be installed around the perimeter of the site and will collect stormwater runoff from adjacent areas and the gravel access road (this excludes the maintenance access route for the stormwater features, which will not be utilized with sufficient frequency to require pollutant mitigation measures). These swales will then discharge to a sediment forebay featuring a downstream check dam. Based on the HydroCAD model, flow within the swales is less than 2" deep, with velocities lower than 1 ft/s and total travel times in excess of 9 minutes during the 1" water quality design storm event. Therefore, based on the Massachusetts Stormwater Handbook for grassed channels, the swale should be suitable to provide effective pretreatment of TSS (50%), which will be combined with additional treatment by the sediment forebay downstream.

Dielectric Fluid Containment Areas, Inhibition Device, and Oil/Water Separator

Multiple oil absorbing inhibition devices will be employed for multiple layers of defense against dielectric fluid release from the dielectric fluid containment structures. Each electrical component containment will be piped to a common drain header. Immediately downstream of each individual sump will be an oil absorbing inhibition device located in a well below the frost line. The oil absorbing inhibition device consists of oil absorbing resin that swells and blocks flow when the dielectric fluid is found but allows rainwater to drain through. The header that collects the rainwater leads to an oil water separator (to remove any sheen), followed by another inhibition device for an added factor of safety.

Inhibition devices are commonly used. Brand names include Imbiber, C.I. Agent, HFF valve or equals. Final selection of an individual manufacturer has not been made at this time. These devices are manufactured in various forms including a filter like assembly, and in line piping assembly. Functionally they perform similarly. Final selection will be based on durability, capacity, ease of maintenance, and proven history of performance.

An additional tank oil/water separator system is proposed downstream of the above devices to help capture residual contaminants. These systems work on a hydrological difference between the inlet and outlet to allow for the dielectric fluid to rise in a prefabricated chamber. This allows the dielectric fluid to become trapped on the surface before being removed during normal maintenance operations. These systems also utilize screens and coalescers to capture the dielectric fluids and other sinking debris in the water.

1.6 Comments on Nitrogen Control Issues

Following redevelopment, the site will not contribute nitrogen (N) through wastewater. Only portable restrooms are to be used during construction and operation of the substation. Also, the substation site will comply with the Massachusetts Stormwater Policy and employ low impact development strategies to minimize stormwater runoff, and treat any runoff generated from paved areas (main access road and parking) prior to recharge to the ground. The maintenance access road will be not be utilized frequently and is therefore not a substantial source of nitrogen. The Nitrogen loading model presented in the Cape Cod Commission Nitrogen Loading Technical Bulletin 91-001, has been used to evaluate potential N loading from the substation site. A calculation based on this document is appended in Section 6, which shows that the projected N loading will be 0.35 ppm - less than the 1 ppm standard set by the Cape Cod Commission.

1.7 Analysis Summary

Detailed HydroCAD calculations of the stormwater drainage conditions for the design storm events are submitted in Section 2. As noted in the summary table below, the post-development total discharge volumes and peak runoff rates from the internal depression area remain 0 cfs, even during the most severe modelled design storm (1 in 100 year).

Design Point: North-eastern Internal Depression

Table 1. Summary Table

		Peak Runoff Rate (cfs)	Total Runoff Volume (af)
100-year Storm Event:	Existing Conditions	0	0
	Proposed Conditions	0	0

* See discussion below. cfs = cubic feet per second; af = acre feet

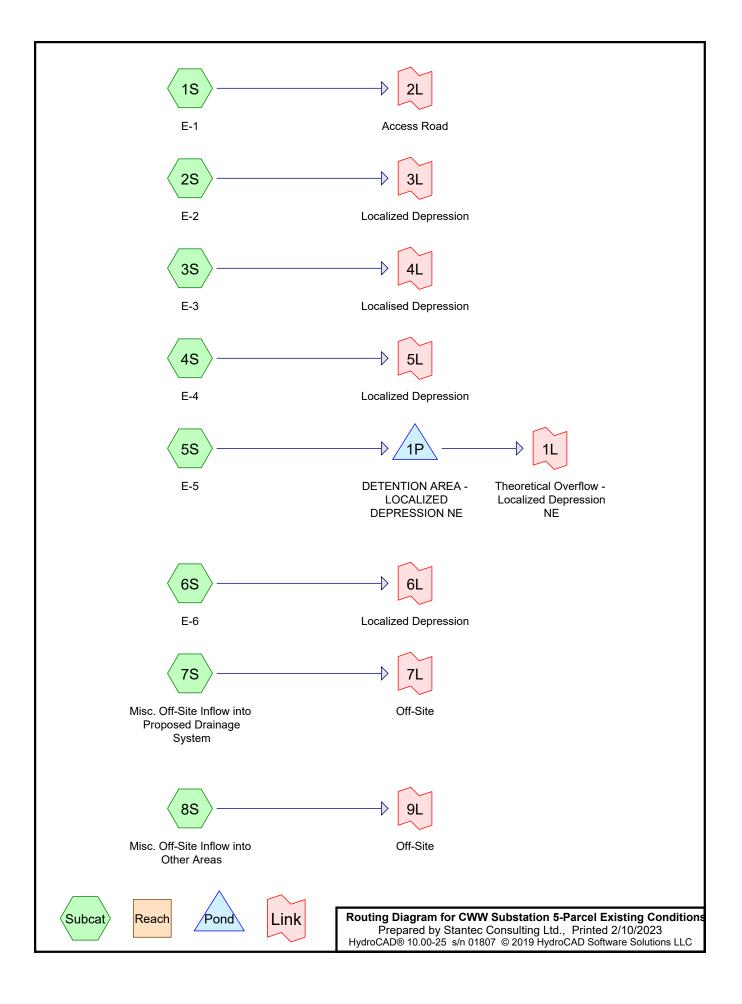
Overall, the proposed stormwater management design will meet or exceed the Massachusetts Stormwater Policy recommendations for this project; and the project will comply with the MassDEP Stormwater Standards. A summary of project status with respect to each Standard is as follows:

- Standard 1 No New Untreated Discharges. This Standard will be met; and no new stormwater conveyances would discharge untreated stormwater directly to the waters of the Commonwealth.
- **Standard 2 Peak Rate Attenuation**. This Standard will be met. Post-development peak discharge runoff rates will not exceed the pre-development rates.
- **Standard 3 Recharge.** This Standard will be met; and the recharge volume required by the Policy for this project will be met or exceeded.
- **Standard 4 Water Quality.** This Standard will be met; and the project will meet the required water quality standards. The proposed design will remove at least 44% of TSS prior to discharge to an infiltration structure as required per Stormwater Policy.
- Standard 5 Land Uses with Higher Potential Pollutant Loads. This Standard does not apply to this project. However, as noted, surplus containment areas will be provided for any substation equipment that contains dielectric fluid.
- Standard 6 Critical Areas. The site does not lie within Zones I or II or within the Interim Wellhead Protection area of a public water supply, and there are no stormwater discharges near or to any other critical area.

- Standard 7 Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable. The project is not defined as a redevelopment in the Massachusetts Stormwater Management Standards so all Standards must be met.
- Standard 8 Construction Period Pollution Prevention and Erosion and Sedimentation Control. An Erosion & Sedimentation control plan will be prepared for the prevention of erosion, sedimentation, and off-site transport of suspended solids; and a draft of this plan is included in Section Five of this Stormwater Report.
- Standard 9 Operation and Maintenance Plan. A Long-Term Operation and Maintenance Plan will be prepared as part of final design.
- Standard 10 Prohibition of Illicit Discharges. Per Standard No. 10 of the MassDEP Stormwater Management Standards, there shall be no illicit discharges to the stormwater management system. The Property Manager will be responsible for implementing the Operation and Maintenance Plan for the Site's stormwater management system; and for overseeing activities at the facility to prevent illicit discharges to the drainage system. It is strictly prohibited to discharge any products or substances onto the ground surface or into any drainage structures, such as catch basin inlets, manholes, or drainage outlets that would be a detriment to the environment.

Section 2

Analysis for 2-, 10-, 50- and 100-year Storms Existing Conditions HydroCAD Analysis Proposed Conditions HydroCAD Analysis



Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
2,020	96	Gravel surface, HSG A (1S)
1,039,500	30	Woods, Good, HSG A (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
1,041,520	30	TOTAL AREA

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
1,041,520	HSG A	1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S
0	HSG B	
0	HSG C	
0	HSG D	
0	Other	
1,041,520		TOTAL AREA

CWW Substation 5-Parcel Existing Conditions	
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				-			
HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
 (sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover	Numbers
2,020	0	0	0	0	2,020	Gravel surface	1
							S
1,039,500	0	0	0	0	1,039,500	Woods, Good	1
							S,
							2
							2 S,
							З,
							3
							S,
							4
							S,
							5
							5 S,
							З,
							6
							S,
							7
							S,
							<u> </u>
							8
	•		•	•			S
1,041,520	0	0	0	0	1,041,520	TOTAL AREA	

Ground Covers (all nodes)

CWW Substation 5-Parcel Existing Conditi Type III 24-hr NRCC 100YR 24H Rainfall=8.38"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 5

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: E-1	Runoff Area=27 ow Length=285'			
Subcatchment 2S: E-2	Runoff Area=45 ow Length=230'			
Subcatchment 3S: E-3	Runoff Area=68 ow Length=356'	•	•	
Subcatchment 4S: E-4	Runoff Area=134 ow Length=450'			
Subcatchment 5S: E-5 Flow	Runoff Area=547 Length=1,300' 1			
Subcatchment 6S: E-6	Runoff Area=182 ow Length=552'	•	•	
Subcatchment7S: Misc. Off-Site Inflow into	Runoff Area=4).00% Imper) min CN=3	•	
Subcatchment8S: Misc. Off-Site Inflow into	Runoff Area=31).00% Imper nin CN=30		
Pond 1P: DETENTION AREA - LOCALIZED Discarded=0.3	Peak Elev=85. 9 cfs 9,126 cf F			
Link 1L: Theoretical Overflow - Localized D	epression NE		flow=0.00 nary=0.00	
Link 2L: Access Road			=0.29 cfs =0.29 cfs	
Link 3L: Localized Depression			=0.16 cfs =0.16 cfs	
Link 4L: Localised Depression			=0.24 cfs =0.24 cfs	
Link 5L: Localized Depression			=0.47 cfs =0.47 cfs	
Link 6L: Localized Depression			=0.63 cfs =0.63 cfs	
Link 7L: Off-Site			w=0.02 cfs y=0.02 cfs	

Link 9L: Off-Site

Inflow=0.13 cfs 1,119 cf Primary=0.13 cfs 1,119 cf

Total Runoff Area = 1,041,520 sf Runoff Volume = 36,697 cf Average Runoff Depth = 0.42" 100.00% Pervious = 1,041,520 sf 0.00% Impervious = 0 sf CWW Substation 5-Parcel Existing Conditi Type III 24-hr NRCC 100YR 24H Rainfall=8.38"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 7

Summary for Subcatchment 1S: E-1

Runoff = 0.29 cfs @ 12.29 hrs, Volume= 1,811 cf, Depth> 0.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

_	A	rea (sf)	CN I	Description		
		24,980	30	Woods, Go	od, HSG A	
_		2,020	96	Gravel surfa	ace, HSG A	A
		27,000	35	Weighted A	verage	
		27,000		100.00% Pe	ervious Are	a
	Тс	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.8	50	0.2000	0.17		Sheet Flow, SHEET
						Woods: Light underbrush n= 0.400 P2= 3.29"
	6.1	235	0.0650	0.64		Shallow Concentrated Flow, SHALLOW CONC
_						Forest w/Heavy Litter Kv= 2.5 fps
	10.9	285	Total			

Summary for Subcatchment 2S: E-2

Runoff = 0.16 cfs @ 12.48 hrs, Volume= 1,574 cf, Depth> 0	Runoff
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

Are	a (sf)	CN E	Description		
4:	5,304	30 V	Voods, Go	od, HSG A	
4	5,304	1	00.00% Pe	ervious Are	a
Tc L (min)	_ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.4	50	0.0700	0.11		Sheet Flow, SHEET
4.9	180	0.0600	0.61		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
12.3	230	Total			

Summary for Subcatchment 3S: E-3

Runoff = 0.24 cfs @ 12.50 hrs, Volume= 2,389 cf, Depth> 0.42"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38" CWW Substation 5-Parcel Existing Conditi Type III 24-hr NRCC 100YR 24H Rainfall=8.38" Prepared by Stantec Consulting Ltd. Printed 2/10/2023 HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLC Page 8

A	rea (sf)	CN E	escription		
	68,819	30 V	Voods, Go	od, HSG A	
	68,819	1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.4	50	0.1000	0.13		Sheet Flow, Sheet
7.2	306	0.0800	0.71		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
13.6	356	Total			

Summary for Subcatchment 4S: E-4

Runoff = 0.47 cfs @ 12.52 hrs, Volume= 4,675 cf, Depth> 0.42"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

A	rea (sf)	CN E	escription		
1	34,903	30 V	Voods, Go	od, HSG A	
1	34,903	1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.4	50	0.0500	0.10		Sheet Flow, SHEET
6.7	400	0.1600	1.00		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
15.1	450	Total			

Summary for Subcatchment 5S: E-5

Runoff = 1.48 cfs @ 12.77 hrs, Volume= 18,669 cf, Depth> 0.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

Are	ea (sf)	CN D	escription		
54	7,239	30 V	Voods, Go	od, HSG A	
54	7,239	1	00.00% Pe	ervious Are	а
Tc((min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.8	50	0.0600	0.11		Sheet Flow, SHEET
23.1	1,250	0.1300	0.90		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
30.9	1,300	Total			

Summary for Subcatchment 6S: E-6

Runoff = 0.63 cfs @ 12.52 hrs, Volume= 6,313 cf, Depth> 0.42"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

A	rea (sf)	CN E	Description		
1	82,095	30 V	Voods, Go	od, HSG A	
1	82,095	1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.8	50	0.1300	0.14		Sheet Flow, SHEET
8.9	502	0.1400	0.94		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
14.7	552	Total			

Summary for Subcatchment 7S: Misc. Off-Site Inflow into Proposed Drainage System

146 cf, Depth> 0.42"

Runoff = 0.02 cfs @ 12.37 hrs, Volume=

5.0

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

Area (sf)	CN Description
4,170	30 Woods, Good, HSG A
4,170	100.00% Pervious Area
Tc Length (min) (feet)	
5.0	Direct Entry, Direct Entry
Sum	mary for Subcatchment 8S: Misc. Off-Site Inflow into Other Areas
Runoff =	0.13 cfs @ 12.37 hrs, Volume= 1,119 cf, Depth> 0.42"
	R-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs RCC 100YR 24H Rainfall=8.38"
Area (sf)	CN Description
31,990	30 Woods, Good, HSG A
31,990	100.00% Pervious Area
Tc Length (min) (feet)	

Direct Entry, Direct Entry

Summary for Pond 1P: DETENTION AREA - LOCALIZED DEPRESSION NE

Inflow Area =	547,239 sf, 0.00% Impervious, Inflow Depth > 0.41" for NRCC 100YR 24H event
Inflow =	1.48 cfs @ 12.77 hrs, Volume= 18,669 cf
Outflow =	0.39 cfs @ 18.32 hrs, Volume= 9,126 cf, Atten= 73%, Lag= 332.9 min
Discarded =	0.39 cfs @ 18.32 hrs, Volume= 9,126 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 85.67' @ 18.32 hrs Surf.Area= 7,058 sf Storage= 9,732 cf

Plug-Flow detention time= 189.7 min calculated for 9,096 cf (49% of inflow) Center-of-Mass det. time= 82.8 min (1,000.5 - 917.6)

Volume	Inver	t Avail.Sto	rage	Storage D	Description	
#1	83.00	122,20)9 cf	Custom S	Stage Data (P	rismatic)Listed below (Recalc)
Elevatio		urf.Area		Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic	c-feet)	(cubic-feet)	
83.0	00	827		0	0	
84.0	00	2,756		1,792	1,792	
85.0	00	5,016		3,886	5,678	
86.0	00	8,057		6,537	12,214	
87.0	00	12,347	1	0,202	22,416	
88.0	00	17,968	1	5,158	37,574	
89.0		24,950		1,459	59,033	
90.0	00	31,470	2	8,210	87,243	
91.0	00	38,463	3	4,967	122,209	
Device	Routing	Invert	Outle	et Devices		
#1	Discarded	83.00'	2.410) in/hr Exf	filtration over	Surface area
#2	Primary	91.90'	50.0'	long x 1.	0' breadth Br	oad-Crested Rectangular Weir
	2					0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50	3.00		
			Coef	. (English)	2.69 2.72 2.	75 2.85 2.98 3.08 3.20 3.28 3.31
			3.30	3.31 3.32	2	
Discard		Max=0.39 cf			W=85.67' (Fr	ee Discharge)

1=Exfiltration (Exfiltration Controls 0.39 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=83.00' (Free Discharge)

Summary for Link 1L: Theoretical Overflow - Localized Depression NE

Inflow Are	ea =	547,239 sf,	0.00% Impervious,	Inflow Depth = 0.00" for NRCC 100YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atten= 0%, Lag= 0.0 min

Summary for Link 2L: Access Road

 Inflow Area =
 27,000 sf,
 0.00% Impervious,
 Inflow Depth >
 0.80"
 for
 NRCC 100YR 24H event

 Inflow =
 0.29 cfs @
 12.29 hrs,
 Volume=
 1,811 cf

 Primary =
 0.29 cfs @
 12.29 hrs,
 Volume=
 1,811 cf,

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 3L: Localized Depression

Inflow Are	a =	45,304 sf, 0.00% Impervious, Inflow Depth > 0.42" for NRCC 100YR 24H event
Inflow	=	0.16 cfs @ 12.48 hrs, Volume= 1,574 cf
Primary	=	0.16 cfs @ 12.48 hrs, Volume= 1,574 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 4L: Localised Depression

Inflow Are	a =	68,819 sf, 0.00% Impervious	Inflow Depth > 0.42" for NRCC 100YR 24H event
Inflow	=	0.24 cfs @ 12.50 hrs, Volume=	2,389 cf
Primary	=	0.24 cfs @ 12.50 hrs, Volume=	2,389 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 5L: Localized Depression

Inflow Are	a =	134,903 sf, 0.00% Imperv	/ious, Inflow Depth >	0.42"	for NRCC 100YR 24H event
Inflow	=	0.47 cfs @ 12.52 hrs, Volu	ime= 4,675 d	of	
Primary	=	0.47 cfs @ 12.52 hrs, Volu	ime= 4,675 d	of, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 6L: Localized Depression

Inflow Are	a =	182,095 sf, 0.00% Impervio	ous, Inflow Depth > 0.42" for	NRCC 100YR 24H event
Inflow	=	0.63 cfs @ 12.52 hrs, Volum	ne= 6,313 cf	
Primary	=	0.63 cfs @ 12.52 hrs, Volum	ne= 6,313 cf, Atten= 0%	5, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 7L: Off-Site

Inflow Are	a =	4,170 sf, 0.00% Imperviou	s, Inflow Depth > 0.42" for NRCC 100YR 24H event
Inflow	=	0.02 cfs @ 12.37 hrs, Volume	= 146 cf
Primary	=	0.02 cfs @ 12.37 hrs, Volume	= 146 cf, Atten= 0%, Lag= 0.0 min

Summary for Link 9L: Off-Site

 Inflow Area =
 31,990 sf,
 0.00% Impervious,
 Inflow Depth >
 0.42"
 for
 NRCC 100YR 24H event

 Inflow =
 0.13 cfs @
 12.37 hrs,
 Volume=
 1,119 cf

 Primary =
 0.13 cfs @
 12.37 hrs,
 Volume=
 1,119 cf,

CWW Substation 5-Parcel Existing Conditio Type III 24-hr NRCC 10YR 24H Rainfall=4.83"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 13

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: E-1	Runoff Area=27,000 sf 0.00% Impervious Runoff Depth>0.04" Flow Length=285' Tc=10.9 min CN=35 Runoff=0.00 cfs 93 cf
Subcatchment2S: E-2	Runoff Area=45,304 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=230' Tc=12.3 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment3S: E-3	Runoff Area=68,819 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=356' Tc=13.6 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment4S: E-4	Runoff Area=134,903 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=450' Tc=15.1 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment 5S: E-5	Runoff Area=547,239 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=1,300' Tc=30.9 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment6S: E-6	Runoff Area=182,095 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=552' Tc=14.7 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment7S: Misc. Off-Site Inflow into	Runoff Area=4,170 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment8S: Misc. Off-Site Inflow into	Runoff Area=31,990 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=30 Runoff=0.00 cfs 0 cf
Pond 1P: DETENTION AREA - LOCALIZED Disca	Peak Elev=83.00' Storage=0 cf Inflow=0.00 cfs 0 cf rded=0.00 cfs 0 cf Primary=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf
Link 1L: Theoretical Overflow - Localized D	epression NE Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Link 2L: Access Road	Inflow=0.00 cfs 93 cf Primary=0.00 cfs 93 cf
Link 3L: Localized Depression	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Link 4L: Localised Depression	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Link 5L: Localized Depression	Inflow=0.00 cfs 0 cf
	Primary=0.00 cfs 0 cf
Link 6L: Localized Depression	

Link 9L: Off-Site

Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

Total Runoff Area = 1,041,520 sf Runoff Volume = 93 cf Average Runoff Depth = 0.00" 100.00% Pervious = 1,041,520 sf 0.00% Impervious = 0 sf

Summary for Subcatchment 1S: E-1

Runoff = 0.00 cfs @ 15.48 hrs, Volume= 93 cf, Depth> 0.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

	A	rea (sf)	CN	Description		
		24,980	30	Woods, Go	od, HSG A	
		2,020	96	Gravel surfa	ace, HSG A	١
		27,000		Weighted A		
		27,000		100.00% P	ervious Are	a
	Тс	Length	Slope		Capacity	Description
(m	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.8	50	0.2000	0.17		Sheet Flow, SHEET
						Woods: Light underbrush n= 0.400 P2= 3.29"
	6.1	235	0.0650	0.64		Shallow Concentrated Flow, SHALLOW CONC
						Forest w/Heavy Litter Kv= 2.5 fps
1	0.9	285	Total			

Summary for Subcatchment 2S: E-2

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

	A	rea (sf)	CN E	Description				
		45,304 30 Woods, Good, HSG A						
		45,304	1	00.00% Pe	ervious Are	a		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
_	7.4	50	0.0700	0.11		Sheet Flow, SHEET		
	4.9	180	0.0600	0.61		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps		
	12.3	230	Total					

Summary for Subcatchment 3S: E-3

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83" CWW Substation 5-Parcel Existing Conditio Type III 24-hr NRCC 10YR 24H Rainfall=4.83"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 16

A	rea (sf)	CN E	Description		
	68,819	30 V	Voods, Go	od, HSG A	
	68,819	1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.4	50	0.1000	0.13		Sheet Flow, Sheet
7.2	306	0.0800	0.71		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
13.6	356	Total			

Summary for Subcatchment 4S: E-4

Runoff	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Depth= 0.00"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

ΑΑ	rea (sf)	CN E	Description		
1	34,903	30 V	Voods, Go	od, HSG A	
1	34,903	1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.4	50	0.0500	0.10		Sheet Flow, SHEET
6.7	400	0.1600	1.00		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
15.1	450	Total			

Summary for Subcatchment 5S: E-5

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

A	rea (sf)	CN E	Description		
5	47,239	30 V	Voods, Go	od, HSG A	
5	547,239 100.00% Pervious Ar			ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.8	50	0.0600	0.11		Sheet Flow, SHEET
23.1	1,250	0.1300	0.90		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
30.9	1,300	Total			

Summary for Subcatchment 6S: E-6

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

A	rea (sf)	CN D	escription		
1	82,095	30 V	Voods, Go	od, HSG A	
1	82,095	1	00.00% Pe	ervious Are	а
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.8	50	0.1300	0.14		Sheet Flow, SHEET
8.9	502	0.1400	0.94		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
14.7	552	Total			

Summary for Subcatchment 7S: Misc. Off-Site Inflow into Proposed Drainage System

Runoff = 0.00 cfs @ 5.00 hrs, Volume=

0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

A	rea (sf)	CN E	Description								
	4,170	,170 30 Woods, Good, HSG A									
	4,170	1	00.00% P	ervious Are	а						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
5.0					Direct Entry, I	Direct Entry					
	Summary for Subcatchment 8S: Misc. Off-Site Inflow into Other Areas										
Runoff	=	0.00 cf	s@ 5.0	0 hrs, Volu	ime=	0 cf, Depth= 0.00"					
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"										

Are	a (sf)	CN E	Description					
31	1,990	30 V	Woods, Good, HSG A					
31	1,990	1	00.00% Pe	ervious Are	a			
Tc L (min)	ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry, Direct Entry			

Summary for Pond 1P: DETENTION AREA - LOCALIZED DEPRESSION NE

Inflow Area =	547,239 sf,	0.00% Impervious,	Inflow Depth = 0.00" for NRCC 10YR 24H event
Inflow =	0.00 cfs @	5.00 hrs, Volume=	0 cf
Outflow =	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atten= 0%, Lag= 0.0 min
Discarded =	0.00 cfs @	5.00 hrs, Volume=	0 cf
Primary =	0.00 cfs @	5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 83.00' @ 5.00 hrs Surf.Area= 827 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)

Volume	Inver	t Avail.Sto	rage S	Storage D	escription			
#1	83.00	' 122,20	09 cf 🕻	Custom S	tage Data (P	rismatic)Listed below (Recalc)		
Elevatio	n S	urf.Area	Inc.S	tore	Cum.Store			
(fee		(sq-ft)	(cubic-1		(cubic-feet)			
83.0		827		0	0			
84.0	00	2,756	1	,792	1,792			
85.0	00	5,016	3	,886	5,678			
86.0		8,057		,537	12,214			
87.0		12,347		,202	22,416			
88.0		17,968		,158	37,574			
89.0		24,950		,459	59,033			
90.0		31,470		,210	87,243			
91.0	00	38,463	34	,967	122,209			
Device	Routing	Invert	Outlet	Devices				
#1	Discarded	83.00'	2.410	in/hr Exfi	Itration over	Surface area		
#2	Primary	91.90'	50.0' l	ong x 1.0	0' breadth Br	oad-Crested Rectangular Weir		
				· /	0 0.40 0.60	0.80 1.00 1.20 1.40 1.60 1.80 2.00		
			2.50 3					
				· · ·		75 2.85 2.98 3.08 3.20 3.28 3.31		
			3.30	3.31 3.32				
	Discarded OutFlow Max=0.00 cfs @ 5.00 hrs HW=83.00' (Free Discharge)							

—1=Exfiltration (Passes 0.00 cfs of 0.05 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=83.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link 1L: Theoretical Overflow - Localized Depression NE

Inflow Are	ea =	547,239 sf,	0.00% Impervious,	Inflow Depth = 0.00" for NRCC 10YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atten= 0%, Lag= 0.0 min

Summary for Link 2L: Access Road

Inflow Are	a =	27,000 sf,	0.00% Impervious,	Inflow Depth >	0.04"	for NRCC 10YR 24H event
Inflow	=	0.00 cfs @	15.48 hrs, Volume=	93 c	cf	
Primary	=	0.00 cfs @	15.48 hrs, Volume=	93 c	of, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 3L: Localized Depression

Inflow Are	a =	45,304 sf,	0.00% Impervious,	Inflow Depth = 0.00" for NRCC 10YR 24H event	
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atten= 0%, Lag= 0.0 min	

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 4L: Localised Depression

Inflow Are	ea =	68,819 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for NRCC 10YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 5L: Localized Depression

Inflow Are	ea =	134,903 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for NRCC 10YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 6L: Localized Depression

Inflow Are	ea =	182,095 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for NRCC 10YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atter	ו= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 7L: Off-Site

Inflow Are	ea =	4,170 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for NRCC 10YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Summary for Link 9L: Off-Site

Inflow Area =31,990 sf,0.00% Impervious, Inflow Depth =0.00" for NRCC 10YR 24H eventInflow =0.00 cfs @5.00 hrs, Volume=0 cfPrimary =0.00 cfs @5.00 hrs, Volume=0 cf, Atten= 0%, Lag= 0.0 min

CWW Substation 5-Parcel Existing Condition Type III 24-hr NRCC 2YR 24H Rainfall=3.29"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 21

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: E-1	Runoff Area=27,000 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=285' Tc=10.9 min CN=35 Runoff=0.00 cfs 0 cf
Subcatchment2S: E-2	Runoff Area=45,304 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=230' Tc=12.3 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment3S: E-3	Runoff Area=68,819 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=356' Tc=13.6 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment4S: E-4	Runoff Area=134,903 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=450' Tc=15.1 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment 5S: E-5	Runoff Area=547,239 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=1,300' Tc=30.9 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment6S: E-6	Runoff Area=182,095 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=552' Tc=14.7 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment7S: Misc. Off-Site Inflow into	Runoff Area=4,170 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment8S: Misc. Off-Site Inflow into	Runoff Area=31,990 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=30 Runoff=0.00 cfs 0 cf
Pond 1P: DETENTION AREA - LOCALIZED Disca	Peak Elev=83.00' Storage=0 cf Inflow=0.00 cfs 0 cf rded=0.00 cfs 0 cf Primary=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf
Link 1L: Theoretical Overflow - Localized D	epression NE Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Link 2L: Access Road	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Link 3L: Localized Depression	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Link 4L: Localised Depression	Inflow=0.00 cfs_0 cf Primary=0.00 cfs_0 cf
Link 5L: Localized Depression	Inflow=0.00 cfs_0 cf Primary=0.00 cfs_0 cf
Link 6L: Localized Depression	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Link 7L: Off-Site	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

Link 9L: Off-Site

Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

Total Runoff Area = 1,041,520 sf Runoff Volume = 0 cf Average Runoff Depth = 0.00" 100.00% Pervious = 1,041,520 sf 0.00% Impervious = 0 sf

CWW Substation 5-Parcel Existing Condition Type III 24-hr	NRCC 2YR 24H Rainfall=3.29"
Prepared by Stantec Consulting Ltd.	Printed 2/10/2023
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Summary for Subcatchment 1S: E-1

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

 A	rea (sf)	CN	Description						
	24,980	30	Woods, Go	Voods, Good, HSG A					
	2,020	96	Gravel surface, HSG A						
	27,000	35	Weighted A	verage					
	27,000		100.00% P	ervious Are	a				
Тс	Length	Slope	,	Capacity	Description				
 <u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
4.8	50	0.2000	0.17		Sheet Flow, SHEET				
					Woods: Light underbrush n= 0.400 P2= 3.29"				
6.1	235	0.0650	0.64		Shallow Concentrated Flow, SHALLOW CONC				
					Forest w/Heavy Litter Kv= 2.5 fps				
 10.9	285	Total							

Summary for Subcatchment 2S: E-2

Runoff	=	0 00 cfs @	5.00 hrs, Volume=	0 cf, Depth= 0.00"
runon		0.00 013 @	0.00 m3, volume=	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

	Area (sf)	CN [Description						
	45,304	30 \	0 Woods, Good, HSG A						
	45,304		100.00% P	ervious Are	a				
T (mir	c Length	Slope (ft/ft)	,	Capacity (cfs)	Description				
7.	4 50	0.0700			Sheet Flow, SHEET				
4.	9 180	0.0600	0.61		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps				
12.	3 230	Total							

Summary for Subcatchment 3S: E-3

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29" CWW Substation 5-Parcel Existing Condition Type III 24-hr NRCC 2YR 24H Rainfall=3.29"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 24

A	rea (sf)	CN E	Description		
	68,819	30 V	Voods, Go	od, HSG A	
	68,819	1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.4	50	0.1000	0.13		Sheet Flow, Sheet
7.2	306	0.0800	0.71		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
13.6	356	Total			

Summary for Subcatchment 4S: E-4

Runoff	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Depth= 0.00"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

A	rea (sf)	CN E	escription			
1	134,903		Woods, Good, HSG A			
134,903		1	00.00% Pe	ervious Are	a	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
8.4	50	0.0500	0.10		Sheet Flow, SHEET	
6.7	400	0.1600	1.00		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps	
15.1	450	Total				

Summary for Subcatchment 5S: E-5

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

A	rea (sf)	CN E	Description				
547,239		30 V) Woods, Good, HSG A				
547,239		1	00.00% Pe	ervious Are	a		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
7.8	50	0.0600	0.11		Sheet Flow, SHEET		
23.1	1,250	0.1300	0.90		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps		
30.9	1,300	Total					

Summary for Subcatchment 6S: E-6

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

A	rea (sf)	CN E	Description		
1	82,095	30 V	Voods, Go	od, HSG A	
1	82,095	100.00% Pervious Are			a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.8	50	0.1300	0.14		Sheet Flow, SHEET
8.9	502	0.1400	0.94		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
14.7	552	Total			

Summary for Subcatchment 7S: Misc. Off-Site Inflow into Proposed Drainage System

0 cf, Depth= 0.00"

Runoff = 0.00 cfs @ 5.00 hrs, Volume=

Area (s	sf) CN	Description					
4,1	70 30	Woods, Go	od, HSG A				
4,1	70	100.00% P	ervious Are	a			
Tc Len (min) (fe	•	pe Velocity /ft) (ft/sec)	Capacity (cfs)	Description			
5.0				Direct Entry, Direct Entry			
Su	Summary for Subcatchment 8S: Misc. Off-Site Inflow into Other Areas						
Runoff =	0.00) cfs @ 5.0	0 hrs, Volu	ume= 0 cf, Depth= 0.00"			
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"							
Area (sf) CN	Description					

Area (st)	CN Description				
31,990	30 Woods, Good, HSG A				
31,990	100.00% Pervious Ar	ea			
Tc Length (min) (feet)	Slope Velocity Capacity (ft/ft) (ft/sec) (cfs)	Description			
5.0		Direct Entry, Direct Entry			

Summary for Pond 1P: DETENTION AREA - LOCALIZED DEPRESSION NE

Inflow Area =	547,239 sf,	0.00% Impervious,	Inflow Depth = 0.00" for NRCC 2YR 24H event
Inflow =	0.00 cfs @	5.00 hrs, Volume=	0 cf
Outflow =	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atten= 0%, Lag= 0.0 min
Discarded =	0.00 cfs @	5.00 hrs, Volume=	0 cf
Primary =	0.00 cfs @	5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 83.00' @ 5.00 hrs Surf.Area= 827 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)

Volume	Inver	t Avail.Sto	rage S	Storage D	escription	
#1	83.00	' 122,20	09 cf 🕻	Custom S	tage Data (P	rismatic)Listed below (Recalc)
Elevatio	n S	urf.Area	Inc.S	tore	Cum.Store	
(fee		(sq-ft)	(cubic-1		(cubic-feet)	
83.0		827		0	0	
84.0	00	2,756	1	,792	1,792	
85.0	00	5,016	3	,886	5,678	
86.0		8,057		,537	12,214	
87.0		12,347		,202	22,416	
88.0		17,968		,158	37,574	
89.0		24,950		,459	59,033	
90.0		31,470		,210	87,243	
91.0	00	38,463	34	,967	122,209	
Device	Routing	Invert	Outlet	Devices		
#1	Discarded	83.00'	2.410	in/hr Exfi	Itration over	Surface area
#2	Primary	91.90'	50.0' l	ong x 1.0)' breadth Br	oad-Crested Rectangular Weir
	-		Head	(feet) 0.2	0 0.40 0.60	0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3			
				· • /		.75 2.85 2.98 3.08 3.20 3.28 3.31
			3.30 3	3.31 3.32		
Discarded OutFlow Max=0.00 cfs @ 5.00 hrs HW=83.00' (Free Discharge)						

1=Exfiltration (Passes 0.00 cfs of 0.05 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=83.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link 1L: Theoretical Overflow - Localized Depression NE

Inflow Are	a =	547,239 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for NRCC 2YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atte	en= 0%, Lag= 0.0 min

Summary for Link 2L: Access Road

Inflow Area =		27,000 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for NRCC 2YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 3L: Localized Depression

Inflow Are	a =	45,304 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for NRCC 2YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 4L: Localised Depression

Inflow Are	a =	68,819 sf,	0.00% Impervious,	Inflow Depth = $0.00"$	for NRCC 2YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 5L: Localized Depression

Inflow Are	a =	134,903 sf,	0.00% Impervious,	Inflow Depth = $0.00"$	for NRCC 2YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 6L: Localized Depression

Inflow Are	ea =	182,095 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for NRCC 2YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 7L: Off-Site

Inflow Are	ea =	4,170 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for NRCC 2YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atte	n= 0%, Lag= 0.0 min

Summary for Link 9L: Off-Site

Inflow Are	a =	31,990 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for NRCC 2YR 24H event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: E-1 Flo	Runoff Area=2 w Length=285'		•		•	
Subcatchment 2S: E-2 Flo	Runoff Area=4 w Length=230'		•		•	
Subcatchment 3S: E-3 Flo	Runoff Area=6 w Length=356'		•		•	
	Runoff Area=13 w Length=450'					
	Runoff Area=54 .ength=1,300'		•		•	
	Runoff Area=18 w Length=552'		•		•	
Subcatchment7S: Misc. Off-Site Inflow into	Runoff Area=			Runoff [noff=0.0		
Subcatchment8S: Misc. Off-Site Inflow into	Runoff Area=3			Runoff [off=0.11		
Pond 1P: DETENTION AREA - LOCALIZED Discarded=0.37						
Link 1L: Theoretical Overflow - Localized De	pression NE			Inflow=0 rimary=0		
Link 2L: Access Road				w=0.26 ry=0.26		
Link 3L: Localized Depression				w=0.14 ry=0.14		
Link 4L: Localised Depression				w=0.21 ry=0.21		
Link 5L: Localized Depression				w=0.39 ry=0.39		
Link 6L: Localized Depression				w=0.53 ry=0.53		
Link 7L: Off-Site				flow=0.0 hary=0.0		

Link 9L: Off-Site

Inflow=0.11 cfs 1,014 cf Primary=0.11 cfs 1,014 cf

Total Runoff Area = 1,041,520 sf Runoff Volume = 33,256 cf Average Runoff Depth = 0.38" 100.00% Pervious = 1,041,520 sf 0.00% Impervious = 0 sf

Summary for Subcatchment 1S: E-1

Runoff = 0.26 cfs @ 12.33 hrs, Volume= 1,681 cf, Depth> 0.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

_	A	rea (sf)	CN I	Description					
		24,980	30	Woods, Go	od, HSG A				
_		2,020	96	Gravel surface, HSG A					
		27,000	35	Weighted A	verage				
		27,000		100.00% Pe	ervious Are	а			
	Тс	Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	4.8	50	0.2000	0.17		Sheet Flow, SHEET			
						Woods: Light underbrush n= 0.400 P2= 3.29"			
	6.1	235	0.0650	0.64		Shallow Concentrated Flow, SHALLOW CONC			
_						Forest w/Heavy Litter Kv= 2.5 fps			
	10.9	285	Total						

Summary for Subcatchment 2S: E-2

Runoff = 0.14 cfs @ 12.50 hrs, Volume= 1,425 cf, Depth> 0.3	Runoff	=	0.14 cfs @	12.50 hrs.	Volume=	1.425 cf.	Depth>	0.38"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

A	rea (sf)	CN E	Description							
	45,304	30 V	30 Woods, Good, HSG A							
	45,304	1	100.00% Pe	ervious Are	a					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
7.4	50	0.0700	0.11		Sheet Flow, SHEET					
4.9	180	0.0600	0.61		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps					
12.3	230	Total								

Summary for Subcatchment 3S: E-3

Runoff = 0.21 cfs @ 12.52 hrs, Volume= 2,162 cf, Depth> 0.38"

CWW Substation 5-Parcel Existing C Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20" Prepared by Stantec Consulting Ltd. Printed 2/10/2023 HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLC Page 32

A	rea (sf)	CN D	escription		
	68,819	30 V	Voods, Go	od, HSG A	
	68,819	1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.4	50	0.1000	0.13		Sheet Flow, Sheet
7.2	306	0.0800	0.71		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps
13.6	356	Total			

Summary for Subcatchment 4S: E-4

Runoff = 0.39 cfs @ 12.54 hrs, Volume= 4,233 cf, Depth> 0.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

ΑΑ	rea (sf)	CN E	Description						
1	34,903	30 V	Woods, Good, HSG A						
1	34,903	1	00.00% Pe	ervious Are	a				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
8.4	50	0.0500	0.10		Sheet Flow, SHEET				
6.7	400	0.1600	1.00		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps				
15.1	450	Total							

Summary for Subcatchment 5S: E-5

Runoff = 1.25 cfs @ 12.80 hrs, Volume= 16,893 cf, Depth> 0.37"

A	rea (sf)	CN D	escription						
5	47,239	30 V	30 Woods, Good, HSG A						
5	47,239	1	00.00% Pe	ervious Are	a				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
7.8	50	0.0600	0.11		Sheet Flow, SHEET				
23.1	1,250	0.1300	0.90		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps				
30.9	1,300	Total							

Summary for Subcatchment 6S: E-6

Runoff = 0.53 cfs @ 12.53 hrs, Volume= 5,716 cf, Depth> 0.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

A	rea (sf)	CN E	Description						
1	82,095	30 V	30 Woods, Good, HSG A						
1	82,095	1	00.00% Pe	ervious Are	a				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.8	50	0.1300	0.14		Sheet Flow, SHEET				
8.9	502	0.1400	0.94		Woods: Light underbrush n= 0.400 P2= 3.29" Shallow Concentrated Flow, SHALLOW CONC Forest w/Heavy Litter Kv= 2.5 fps				
14.7	552	Total							

Summary for Subcatchment 7S: Misc. Off-Site Inflow into Proposed Drainage System

132 cf, Depth> 0.38"

Runoff = 0.01 cfs @ 12.38 hrs, Volume=

Area (sf) CN Description									
4,170 30 Woods, Good, HSG A									
4,170 100.00% Pervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
5.0 Direct Entry, Direct Entry									
Summary for Subcatchment 8S: Misc. Off-Site Inflow into Other Areas									
Runoff = 0.11 cfs @ 12.38 hrs, Volume= 1,014 cf, Depth> 0.38"									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"									
Area (sf) CN Description									
31,990 30 Woods, Good, HSG A									
31,990 100.00% Pervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
5.0 Direct Entry, Direct Entry									

Summary for Pond 1P: DETENTION AREA - LOCALIZED DEPRESSION NE

Inflow Area =	547,239 sf, 0.00% Impervious, Inf	flow Depth > 0.37" for RMAT 50-YR 24H TIER 3 event
Inflow =	1.25 cfs @ 12.80 hrs, Volume=	16,893 cf
Outflow =	0.37 cfs @ 18.34 hrs, Volume=	8,425 cf, Atten= 71%, Lag= 332.5 min
Discarded =	0.37 cfs @ 18.34 hrs, Volume=	8,425 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 85.51' @ 18.34 hrs Surf.Area= 6,567 sf Storage= 8,631 cf

Plug-Flow detention time= 185.0 min calculated for 8,397 cf (50% of inflow) Center-of-Mass det. time= 79.8 min (1,001.6 - 921.8)

Volume	Invert	t Avail.Sto	rage Storage	Description				
#1	83.00	' 122,20	09 cf Custom	Stage Data (Pr	ismatic)Listed below (Recalc)			
Elevatio	n S	urf.Area	Inc.Store	Cum.Store				
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)				
83.0	1	827	0					
84.0	00	2,756	1,792	1,792				
85.0	0	5,016	3,886	5,678				
86.0		8,057	6,537	12,214				
87.0		12,347	10,202	22,416				
88.0	00	17,968	15,158	37,574				
89.0		24,950	21,459	59,033				
90.0		31,470	28,210	87,243				
91.0	00	38,463	34,967	122,209				
Device	Routing	Invert	Outlet Device	s				
#1	Discarded	83.00'	2.410 in/hr E	xfiltration over	Surface area			
#2	Primary	91.90'	50.0' long x	1.0' breadth Bro	oad-Crested Rectangular Weir			
	2		Head (feet) 0 2.50 3.00	.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60 1.80 2.00			
					75 2.85 2.98 3.08 3.20 3.28 3.31			
Discard	Discarded OutFlow Max=0.37 cfs @ 18.34 hrs HW=85.51' (Free Discharge)							

Liscarded OutFlow Max=0.37 cfs @ 18.34 hrs HW=85.51° (Free Dischatting **1=Exfiltration** (Exfiltration Controls 0.37 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=83.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link 1L: Theoretical Overflow - Localized Depression NE

Inflow Are	ea =	547,239 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atter	ר= 0%, Lag= 0.0 min

Summary for Link 2L: Access Road

 Inflow Area =
 27,000 sf,
 0.00% Impervious,
 Inflow Depth >
 0.75"
 for
 RMAT 50-YR 24H TIER 3 event

 Inflow =
 0.26 cfs @
 12.33 hrs,
 Volume=
 1,681 cf

 Primary =
 0.26 cfs @
 12.33 hrs,
 Volume=
 1,681 cf,

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 3L: Localized Depression

Inflow Are	a =	45,304 sf, 0.00% Impervious, Inflow Depth > 0.38" for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.14 cfs @ 12.50 hrs, Volume= 1,425 cf
Primary	=	0.14 cfs @ 12.50 hrs, Volume= 1,425 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 4L: Localised Depression

Inflow Are	a =	68,819 sf, 0.00% Impervious, Inflow Depth > 0.38" for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.21 cfs @ 12.52 hrs, Volume= 2,162 cf
Primary	=	0.21 cfs @ 12.52 hrs, Volume= 2,162 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 5L: Localized Depression

Inflow Area	a =	134,903 sf, 0.00% Impervious, Inflow Depth > 0.38	for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.39 cfs @ 12.54 hrs, Volume= 4,233 cf	
Primary	=	0.39 cfs @ 12.54 hrs, Volume= 4,233 cf, Att	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 6L: Localized Depression

Inflow Area	a =	182,095 sf, 0.00% Impervious	, Inflow Depth > 0.38" for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.53 cfs @ 12.53 hrs, Volume=	5,716 cf
Primary	=	0.53 cfs @ 12.53 hrs, Volume=	5,716 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link 7L: Off-Site

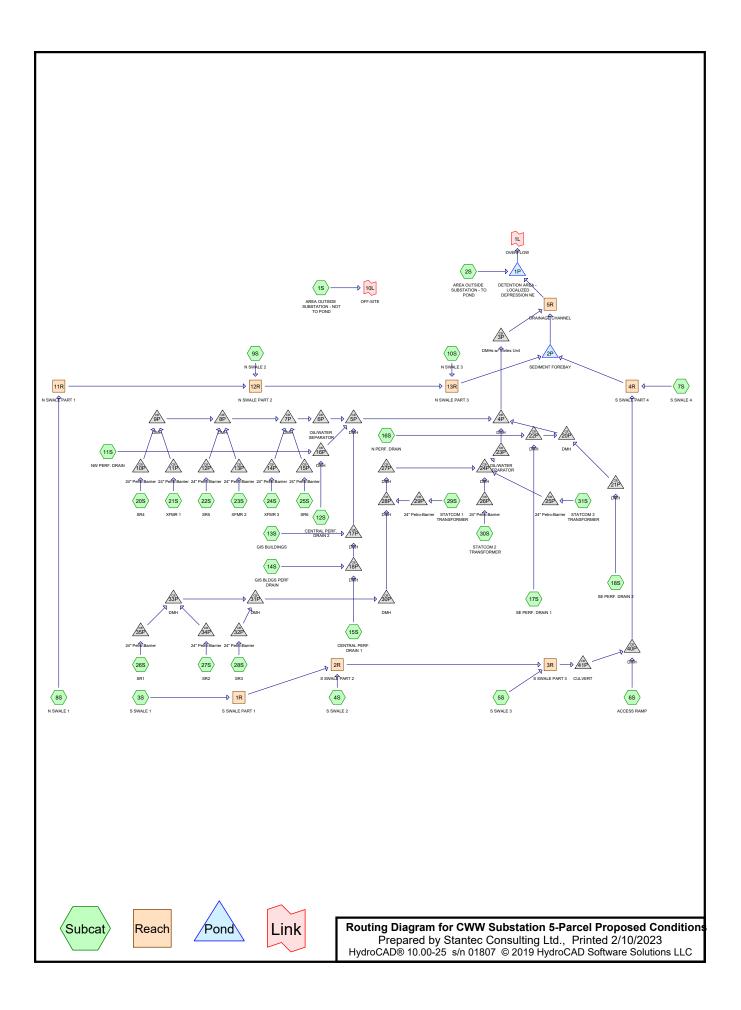
Inflow Area	a =	4,170 sf, 0.00% Im	pervious, Infl	low Depth >	0.38"	for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.01 cfs @ 12.38 hrs, \	volume=	132 cf		
Primary	=	0.01 cfs @ 12.38 hrs, \	√olume=	132 cf	, Atten	= 0%, Lag= 0.0 min

Summary for Link 9L: Off-Site

 Inflow Area =
 31,990 sf,
 0.00% Impervious,
 Inflow Depth >
 0.38"
 for
 RMAT 50-YR 24H TIER 3 event

 Inflow =
 0.11 cfs @
 12.38 hrs,
 Volume=
 1,014 cf

 Primary =
 0.11 cfs @
 12.38 hrs,
 Volume=
 1,014 cf,



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Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
23,427	98	Concrete Containment (20S, 21S, 22S, 23S, 24S, 25S, 26S, 27S, 28S, 29S, 30S, 31S)
302,189	63	Crushed Stone Surface, HSG A (3S, 4S, 5S, 7S, 8S, 9S, 10S, 11S, 12S, 14S, 15S, 16S, 17S, 18S)
76,573	96	Gravel surface, HSG A (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S, 9S, 10S)
183,022	30	Meadow, non-grazed, HSG A (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
34,667	98	Roofs, HSG A (13S, 16S, 17S, 18S)
421,642	30	Woods, Good, HSG A (1S, 2S, 3S, 4S, 5S, 6S)
1,041,520	48	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
1,018,093	HSG A	1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S, 9S, 10S, 11S, 12S, 13S, 14S, 15S, 16S, 17S, 18S
0	HSG B	
0	HSG C	
0	HSG D	
23,427 1,041,520	Other	20S, 21S, 22S, 23S, 24S, 25S, 26S, 27S, 28S, 29S, 30S, 31S TOTAL AREA

CWW Substation 5-Parcel Proposed Conditions

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Printed 2/10/2023 Page 4

	Ground	Total	Other	HSG-D	HSG-C	HSG-B	HSG-A
	Cover	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)
	Concrete	23,427	23,427	0	0	0	0
nt	Containment						
tone	Crushed Stone	302,189	0	0	0	0	302,189
	Surface						
ace	Gravel surface	76,573	0	0	0	0	76,573
	Meadow,	183,022	0	0	0	0	183,022
1	non-grazed						
	Roofs	34,667	0	0	0	0	34,667
od	Woods, Good	421,642	0	0	0	0	421,642
REA	TOTAL AREA	1,041,520	23,427	0	0	0	1,018,093

Ground Covers (all nodes)

Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: AREA OUTSIDE	Runoff Area=258,650 sf 0.00% Impervious Runoff Depth=0.67" Tc=5.0 min CN=32 Runoff=1.69 cfs 14,486 cf
Subcatchment 2S: AREA OUTSIDE	Runoff Area=285,632 sf 0.00% Impervious Runoff Depth=0.59" w Length=491' Tc=12.3 min CN=31 Runoff=1.37 cfs 14,027 cf
Subcatchment 3S: S SWALE 1	Runoff Area=21,901 sf 0.00% Impervious Runoff Depth=3.16"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=56 Runoff=1.80 cfs 5,769 cf
Subcatchment 4S: S SWALE 2	Runoff Area=37,407 sf 0.00% Impervious Runoff Depth=1.52" Tc=5.0 min CN=41 Runoff=1.22 cfs 4,744 cf
Subcatchment 5S: S SWALE 3	Runoff Area=28,531 sf 0.00% Impervious Runoff Depth=2.26"
Flow Length=644'	Slope=0.0200 '/' Tc=9.3 min CN=48 Runoff=1.40 cfs 5,385 cf
Subcatchment 6S: ACCESS RAMP	Runoff Area=17,331 sf 0.00% Impervious Runoff Depth=5.74" Tc=5.0 min CN=78 Runoff=2.74 cfs 8,295 cf
Subcatchment7S: S SWALE 4	Runoff Area=20,291 sf 0.00% Impervious Runoff Depth=3.97"
Flow Length=644'	Slope=0.0200 '/' Tc=9.3 min CN=63 Runoff=1.93 cfs 6,712 cf
Subcatchment8S: N SWALE 1	Runoff Area=15,778 sf 0.00% Impervious Runoff Depth=5.98"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=80 Runoff=2.48 cfs 7,865 cf
Subcatchment9S: N SWALE 2	Runoff Area=14,101 sf 0.00% Impervious Runoff Depth=6.34"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=83 Runoff=2.32 cfs 7,451 cf
Subcatchment 10S: N SWALE 3	Runoff Area=12,739 sf 0.00% Impervious Runoff Depth=6.34"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=83 Runoff=2.10 cfs 6,731 cf
Subcatchment11S: NW PERF. DRAIN	Runoff Area=45,853 sf 0.00% Impervious Runoff Depth=3.97"
Flow Length=660'	Slope=0.0200 '/' Tc=6.0 min CN=63 Runoff=4.88 cfs 15,169 cf
Subcatchment 12S: CENTRAL PERF. DRA	N Runoff Area=6,658 sf 0.00% Impervious Runoff Depth=3.97" Tc=5.0 min CN=63 Runoff=0.73 cfs 2,203 cf
Subcatchment 13S: GIS BUILDINGS	Runoff Area=21,900 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=4.29 cfs 14,855 cf
Subcatchment 14S: GIS BLDGS PERF	Runoff Area=74,801 sf 0.00% Impervious Runoff Depth=3.97"
Flow Length=570'	Slope=0.0200 '/' Tc=8.4 min CN=63 Runoff=7.32 cfs 24,745 cf
	N Runoff Area=54,087 sf 0.00% Impervious Runoff Depth=3.97" Slope=0.0200 '/' Tc=8.4 min CN=63 Runoff=5.29 cfs 17,893 cf
Subcatchment 16S: N PERF. DRAIN	Runoff Area=35,583 sf 0.57% Impervious Runoff Depth=3.97" Tc=5.0 min CN=63 Runoff=3.92 cfs 11,771 cf

Subcatchment17S: SE PERF. DRAIN 1 Flow Length=425'	Runoff Area=41,546 sf 20.00% Impervious Runoff Depth=4.79" Slope=0.0200 '/' Tc=5.9 min CN=70 Runoff=5.38 cfs 16,593 cf
Subcatchment 18S: SE PERF. DRAIN 2	Runoff Area=25,304 sf 16.82% Impervious Runoff Depth=4.67" Tc=5.0 min CN=69 Runoff=3.30 cfs 9,857 cf
Subcatchment20S: SR4	Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.34 cfs 1,184 cf
Subcatchment21S: XFMR 1	Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.53 cfs 1,827 cf
Subcatchment22S: SR5	Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.34 cfs 1,184 cf
Subcatchment23S: XFMR 2	Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.53 cfs 1,827 cf
Subcatchment24S: XFMR 3	Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.53 cfs 1,827 cf
Subcatchment25S: SR6	Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.34 cfs 1,184 cf
Subcatchment26S: SR1	Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.31 cfs 1,085 cf
Subcatchment27S: SR2	Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.31 cfs 1,085 cf
Subcatchment28S: SR3	Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.31 cfs 1,085 cf
Subcatchment29S: STATCOM1	Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.35 cfs 1,201 cf
Subcatchment30S: STATCOM2	Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.35 cfs 1,201 cf
Subcatchment31S: STATCOM3	Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=8.14" Tc=5.0 min CN=98 Runoff=0.35 cfs 1,201 cf
	Avg. Flow Depth=0.50' Max Vel=2.23 fps Inflow=1.80 cfs 5,769 cf 09.0' S=0.0137 '/' Capacity=10.56 cfs Outflow=1.70 cfs 5,769 cf
	vg. Flow Depth=0.57' Max Vel=2.89 fps Inflow=2.88 cfs 10,513 cf 7.3' S=0.0195 '/' Capacity=12.62 cfs Outflow=2.81 cfs 10,513 cf
	vg. Flow Depth=0.69' Max Vel=2.81 fps Inflow=4.20 cfs 15,897 cf 9.0' S=0.0142 '/' Capacity=10.77 cfs Outflow=4.06 cfs 15,897 cf
	vg. Flow Depth=1.00' Max Vel=2.57 fps Inflow=7.94 cfs 30,904 cf 67.8' S=0.0073 '/' Capacity=9.98 cfs Outflow=7.66 cfs 30,904 cf

CWW Substation 5-Parcel Proposed Condit <i>Type III 24-hr</i>	NRCC 100YR 24H Rainfall=8.38"
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Reach 5R: DRAINAGE CHANNEL Avg. Flow Depth=0.42' Max Vel=5.74 fps Inflow=47.83 cfs 165,100 cf n=0.078 L=175.0' S=0.3086 '/' Capacity=198.63 cfs Outflow=47.76 cfs 165,100 cf
Reach 11R: N SWALE PART 1 Avg. Flow Depth=0.55' Max Vel=2.51 fps Inflow=2.48 cfs 7,865 cf n=0.030 L=447.0' S=0.0153 '/' Capacity=11.19 cfs Outflow=2.29 cfs 7,865 cf
Reach 12R: N SWALE PART 2 Avg. Flow Depth=0.71' Max Vel=2.82 fps Inflow=4.52 cfs 15,317 cf n=0.030 L=431.0' S=0.0137 '/' Capacity=10.59 cfs Outflow=4.29 cfs 15,317 cf
Reach 13R: N SWALE PART 3 Avg. Flow Depth=0.82' Max Vel=3.00 fps Inflow=6.20 cfs 22,048 cf n=0.030 L=386.0' S=0.0131 '/' Capacity=10.33 cfs Outflow=5.99 cfs 22,048 cf
Pond 1P: DETENTION AREA - Peak Elev=90.62' Storage=110,462 cf Inflow=47.83 cfs 179,128 cf Discarded=2.16 cfs 178,091 cf Primary=0.33 cfs 1,037 cf Outflow=2.49 cfs 179,128 cf
Pond 2P: SEDIMENT FOREBAY Peak Elev=150.40' Storage=9,809 cf Inflow=13.61 cfs 52,952 cf Discarded=0.14 cfs 16,828 cf Primary=13.35 cfs 36,124 cf Outflow=13.49 cfs 52,953 cf
Pond 3P: DMHs w/ Vortex Unit Peak Elev=150.76' Inflow=38.54 cfs 128,976 cf 36.0" Round Culvert n=0.011 L=25.0' S=0.1200 '/' Outflow=38.54 cfs 128,976 cf
Pond 4P: DMH Peak Elev=153.06' Inflow=38.54 cfs 128,976 cf 36.0" Round Culvert n=0.011 L=36.0' S=0.0639 '/' Outflow=38.54 cfs 128,976 cf
Pond 5P: DMH Peak Elev=158.96' Inflow=24.21 cfs 83,898 cf 24.0" Round Culvert n=0.011 L=263.0' S=0.0186 '/' Outflow=24.21 cfs 83,898 cf
Pond 6P: OIL/WATER SEPARATOR Peak Elev=158.98' Inflow=2.61 cfs 9,033 cf 24.0" Round Culvert n=0.012 L=27.0' S=0.0148 '/' Outflow=2.61 cfs 9,033 cf
Pond 7P: DMH Peak Elev=159.34' Inflow=2.61 cfs 9,033 cf 12.0" Round Culvert n=0.011 L=40.0' S=0.0100 '/' Outflow=2.61 cfs 9,033 cf
Pond 8P: DMH Peak Elev=159.60' Inflow=1.74 cfs 6,022 cf 12.0" Round Culvert n=0.011 L=154.0' S=0.0065 '/' Outflow=1.74 cfs 6,022 cf
Pond 9P: DMH Peak Elev=159.91' Inflow=0.87 cfs 3,011 cf 12.0" Round Culvert n=0.011 L=141.0' S=0.0092 '/' Outflow=0.87 cfs 3,011 cf
Pond 10P: 24" Petro-Barrier Peak Elev=162.81' Inflow=0.34 cfs 1,184 cf 6.0" Round Culvert n=0.010 L=30.0' S=0.0143 '/' Outflow=0.34 cfs 1,184 cf
Pond 11P: 24" Petro-Barrier Peak Elev=160.18' Inflow=0.53 cfs 1,827 cf 6.0" Round Culvert n=0.010 L=18.0' S=0.0122 '/' Outflow=0.53 cfs 1,827 cf
Pond 12P: 24" Petro-Barrier Peak Elev=161.08' Inflow=0.34 cfs 1,184 cf 6.0" Round Culvert n=0.010 L=32.0' S=0.0094 '/' Outflow=0.34 cfs 1,184 cf
Pond 13P: 24" Petro-Barrier Peak Elev=159.81' Inflow=0.53 cfs 1,827 cf 6.0" Round Culvert n=0.010 L=20.0' S=0.0115 '/' Outflow=0.53 cfs 1,827 cf

CWW Substation 5-Parcel Proposed ConditType III 24-hr NRCC 100YR 24H Rainfall=8.38" Prepared by Stantec Consulting Ltd. Printed 2/10/2023 HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLC Page 8 Peak Elev=159.55' Inflow=0.53 cfs 1,827 cf Pond 14P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=21.0' S=0.0133 '/' Outflow=0.53 cfs 1,827 cf Peak Elev=159.44' Inflow=0.34 cfs 1,184 cf Pond 15P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=34.0' S=0.0076 '/' Outflow=0.34 cfs 1,184 cf Peak Elev=161.70' Inflow=5.60 cfs 17,371 cf Pond 16P: DMH 12.0" Round Culvert n=0.011 L=17.0' S=0.0294 '/' Outflow=5.60 cfs 17,371 cf Peak Elev=161.56' Inflow=16.29 cfs 57,493 cf Pond 17P: DMH 24.0" Round Culvert n=0.011 L=88.2' S=0.0442 '/' Outflow=16.29 cfs 57,493 cf Peak Elev=164.45' Inflow=12.61 cfs 42,638 cf Pond 18P: DMH 18.0" Round Culvert n=0.011 L=92.0' S=0.0217 '/' Outflow=12.61 cfs 42,638 cf Pond 20P: DMH Peak Elev=153.87' Inflow=12.58 cfs 38,221 cf 24.0" Round Culvert n=0.011 L=56.0' S=0.0286 '/' Outflow=12.58 cfs 38,221 cf Peak Elev=156.20' Inflow=3.30 cfs 9,857 cf Pond 21P: DMH 12.0" Round Culvert n=0.011 L=62.0' S=0.0410 '/' Outflow=3.30 cfs 9,857 cf Pond 22P: DMH Peak Elev=157.44' Inflow=9.29 cfs 28,364 cf 18.0" Round Culvert n=0.011 L=56.0' S=0.0089 '/' Outflow=9.29 cfs 28,364 cf Peak Elev=153.57' Inflow=1.98 cfs 6,858 cf Pond 23P: OIL/WATER SEPARATOR 12.0" Round Culvert n=0.011 L=182.0' S=0.0093 '/' Outflow=2.01 cfs 6,858 cf Peak Elev=153.81' Inflow=1.98 cfs 6,858 cf Pond 24P: DMH 12.0" Round Culvert n=0.011 L=1.0' S=0.1000 '/' Outflow=1.98 cfs 6,858 cf Peak Elev=153.98' Inflow=0.35 cfs 1,201 cf Pond 25P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=58.0' S=0.0102 '/' Outflow=0.35 cfs 1,201 cf Pond 26P: 24" Petro-Barrier Peak Elev=153.92' Inflow=0.35 cfs 1,201 cf 6.0" Round Culvert n=0.010 L=17.0' S=0.0576 '/' Outflow=0.35 cfs 1.201 cf Peak Elev=154.61' Inflow=1.29 cfs 4,457 cf Pond 27P: DMH 12.0" Round Culvert n=0.011 L=224.0' S=0.0085 '/' Outflow=1.29 cfs 4,457 cf Pond 28P: DMH Peak Elev=155.04' Inflow=1.29 cfs 4,457 cf 12.0" Round Culvert n=0.011 L=50.0' S=0.0080 '/' Outflow=1.29 cfs 4,457 cf Peak Elev=155.34' Inflow=0.35 cfs 1,201 cf Pond 29P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=7.0' S=0.0643 '/' Outflow=0.35 cfs 1,201 cf Peak Elev=160.90' Inflow=0.94 cfs 3,256 cf Pond 30P: DMH 12.0" Round Culvert n=0.011 L=98.0' S=0.0092 '/' Outflow=0.94 cfs 3,256 cf Peak Elev=162.41' Inflow=0.94 cfs 3,256 cf Pond 31P: DMH 12.0" Round Culvert n=0.011 L=232.0' S=0.0060 '/' Outflow=0.94 cfs 3,256 cf Pond 32P: 24" Petro-Barrier Peak Elev=162.80' Inflow=0.31 cfs 1,085 cf 6.0" Round Culvert n=0.010 L=10.0' S=0.0240 '/' Outflow=0.31 cfs 1,085 cf

Pond 33P: DMH	Peak Elev=163.60' Inflow=0.63 cfs 2,171 cf 12.0" Round Culvert n=0.011 L=159.0' S=0.0075 '/' Outflow=0.63 cfs 2,171 cf
Pond 34P: 24" Petro-Barri	er Peak Elev=163.81' Inflow=0.31 cfs 1,085 cf 6.0" Round Culvert n=0.010 L=8.0' S=0.0162 '/' Outflow=0.31 cfs 1,085 cf
Pond 35P: 24" Petro-Barri	er Peak Elev=164.75' Inflow=0.31 cfs 1,085 cf 6.0" Round Culvert n=0.010 L=38.0' S=0.0103 '/' Outflow=0.31 cfs 1,085 cf
Pond 40P: DMH	Peak Elev=159.13' Inflow=6.02 cfs 24,192 cf 24.0" x 12.0" Box Culvert n=0.011 L=10.0' S=0.0300 '/' Outflow=6.02 cfs 24,192 cf
Pond 41P: CULVERT	Peak Elev=159.74' Inflow=4.06 cfs 15,897 cf 24.0" x 12.0" Box Culvert n=0.011 L=55.0' S=0.0182 '/' Outflow=4.06 cfs 15,897 cf
Link 1L: OVERFLOW	Inflow=0.33 cfs 1,037 cf Primary=0.33 cfs 1,037 cf
Link 10L: OFF-SITE	Inflow=1.69 cfs 14,486 cf Primary=1.69 cfs 14,486 cf
Total Dunoff Area	- 4 044 520 of Dunoff Volume - 240 442 of Average Dunoff Donth - 2 42

Total Runoff Area = 1,041,520 sf Runoff Volume = 210,442 cf Average Runoff Depth = 2.42" 94.42% Pervious = 983,426 sf 5.58% Impervious = 58,094 sf

Summary for Subcatchment 1S: AREA OUTSIDE SUBSTATION - NOT TO POND

Runoff = 1.69 cfs @ 12.31 hrs, Volume= 14,486 cf, Depth= 0.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

A	rea (sf)	CN	Description					
1	79,269	30	Woods, Good, HSG A					
	72,994	30	Meadow, no	on-grazed,	HSG A			
	6,387	96	Gravel surfa	ace, HSG A	λ			
2	58,650	32	Weighted A	verage				
2	58,650		100.00% Pe	ervious Are	a			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)				
5.0					Direct Entry, Direct Entry			

Summary for Subcatchment 2S: AREA OUTSIDE SUBSTATION - TO POND

Runoff	=	1.37 cfs @	12.45 hrs,	Volume=	14,027 cf, Depth= 0.59"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

A	rea (sf)	CN [Description		
2	206,991	30 V	Voods, Go	od, HSG A	
	72,294	30 N	Meadow, no	on-grazed,	HSG A
	6,347	96 (Gravel surfa	ace, HSG A	Ι
2	85,632	31 V	Veighted A	verage	
2	85,632	1	100.00% Pe	ervious Are	a
Тс	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.4	50	0.5000	0.25		Sheet Flow, Sheet
					Woods: Light underbrush n= 0.400 P2= 3.29"
8.9	441	0.1100	0.83		Shallow Concentrated Flow, Shallow Conc
					Forest w/Heavy Litter Kv= 2.5 fps
12.3	491	Total			

2.3 491 l otal

Summary for Subcatchment 3S: S SWALE 1

Runoff = 1.80 cfs @ 12.10 hrs, Volume= 5,769 cf, Depth= 3.16"

A	rea (sf)	CN E	Description						
*	3,599	63 C	63 Crushed Stone Surface, HSG A						
	6,746	96 0	Gravel surfa	ace, HSG A	N				
	7,202	30 V	Voods, Go	od, HSG A					
	4,354	30 N	leadow, no	on-grazed,	HSG A				
	21,901	56 V	Veighted A	verage					
	21,901	1	00.00% Pe	ervious Are	a				
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
2.3	50	0.0200	0.36		Sheet Flow, Sheet				
					Fallow n= 0.050 P2= 3.29"				
3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc				
					Nearly Bare & Untilled Kv= 10.0 fps				
6.2	378	Total							
			Summar	y for Sub	catchment 4S: S SWALE 2				
Runoff	=	1.22 cf	s@ 12.0	9 hrs, Volu	Ime= 4,744 cf, Depth= 1.52"				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

	Area (sf)	CN	Description				
	5,160	96	Gravel surface, HSG A Crushed Stone Surface, HSG A				
*	2,578	63					
	20,025	30	Woods, Go	od, HSG A			
	9,644	30	Meadow, no	on-grazed,	HSG A		
	37,407	41	Weighted A	verage			
	37,407		100.00% Pe	ervious Are	а		
(m	Tc Length in) (feet)	Slop (ft/f		Capacity (cfs)	Description		
5	5.0				Direct Entry, Direct Entry		

Summary for Subcatchment 5S: S SWALE 3

Runoff = 1.40 cfs @ 12.14 hrs, Volume= 5,385 cf, Depth= 2.26"

	Area (sf)	CN	Description
*	3,335	63	Crushed Stone Surface, HSG A
	7,061	30	Woods, Good, HSG A
	6,315	96	Gravel surface, HSG A
	11,820	30	Meadow, non-grazed, HSG A
	28,531	48	Weighted Average
	28,531		100.00% Pervious Area

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
						Fallow n= 0.050 P2= 3.29"
	7.0	594	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps
-						Nearly Date & Onlined INV- 10.0 1p3
	9.3	644	Total			

Summary for Subcatchment 6S: ACCESS RAMP

Runoff = 2.74 cfs @ 12.07 hrs, Volume= 8,295 cf, Depth= 5.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

Area (sf)	CN	Description					
12,617	96	Gravel surfa	ace, HSG A	4			
1,094	30	Woods, Go	od, HSG A				
3,620	30	Meadow, no	Meadow, non-grazed, HSG A				
17,331	78	Weighted Average					
17,331		100.00% Pe		a			
Tc Length	Slop	be Velocity	Capacity	Description			
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)				
5.0				Direct Entry, Direct Entry			
				••••••			
		•					

Summary for Subcatchment 7S: S SWALE 4

Runoff = 1.93 cfs @ 12.13 hrs, Volume= 6,712 cf, Depth= 3.97"

_	A	rea (sf)	CN [Description								
*		6,230	63 (Crushed St	rushed Stone Surface, HSG A							
		7,051	96 (Gravel surfa	ace, HSG A	N Contraction of the second seco						
		7,010	30 I	Meadow, no	on-grazed,	HSG A						
		20,291	63 \	Veighted Average								
		20,291		100.00% Pervious Area								
	Тс	Length	Slope		Capacity	Description						
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	2.3	50	0.0200	0.36		Sheet Flow, Sheet						
						Fallow n= 0.050 P2= 3.29"						
	7.0	594	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc						
						Nearly Bare & Untilled Kv= 10.0 fps						
	9.3	644	Total									

Summary for Subcatchment 8S: N SWALE 1

2.48 cfs @ 12.09 hrs, Volume= Runoff = 7,865 cf, Depth= 5.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

_	A	rea (sf)	CN [Description						
*		4,949	63 (Crushed Stone Surface, HSG A						
		9,543	96 (Gravel surfa	ace, HSG A	A				
_		1,286	30 N	Meadow, no	on-grazed,	HSG A				
		15,778	80 \	Veighted A	verage					
15,778 100.00% Pervious Area					а					
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	2.3	50	0.0200	0.36		Sheet Flow, Sheet				
						Fallow n= 0.050 P2= 3.29"				
	3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc				
_						Nearly Bare & Untilled Kv= 10.0 fps				
	6.2	378	Total							

Summary for Subcatchment 9S: N SWALE 2

Runoff 2.32 cfs @ 12.09 hrs, Volume= 7,451 cf, Depth= 6.34" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

	A	rea (sf)	CN [Description						
*		5,475	63 (Crushed Stone Surface, HSG A						
		8,626	96 (Gravel surfa	ace, HSG A	Ν				
		14,101	83 V	Veighted A	verage					
14,101 100.00% Pervious Area				100.00% Pe	ervious Are	а				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	2.3	<u>(1001)</u> 50	0.0200	0.36	(013)	Sheet Flow, Sheet				
	2.0	00	0.0200	0.00		Fallow n= 0.050 P2= 3.29"				
	3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc				
						Nearly Bare & Untilled Kv= 10.0 fps				

6.2 378 Total

Summary for Subcatchment 10S: N SWALE 3

Runoff 2.10 cfs @ 12.09 hrs, Volume= 6,731 cf, Depth= 6.34" =

_	A	rea (sf)	CN [Description		
*		4,958	63 (Crushed St	one Surfac	e, HSG A
_		7,781	96 (Gravel surfa	ace, HSG A	Α
		12,739	83 V	Veighted A	verage	
		12,739	1	00.00% Pe	ervious Are	а
	ŢĊ	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
						Fallow n= 0.050 P2= 3.29"
	3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc
						Nearly Bare & Untilled Kv= 10.0 fps
	6.2	378	Total			

Summary for Subcatchment 11S: NW PERF. DRAIN

Runoff = 4.88 cfs @ 12.09 hrs, Volume= 15,169 cf, Depth= 3.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

	A	rea (sf)	CN E	Description		
*		45,853	63 C	Crushed St	one Surfac	e, HSG A
		45,853	1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
	3.0	255	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps
	0.7	355	0.0200	8.34	6.55	Pipe Channel, Perf Pipe
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
	6.0	660	Total			

Summary for Subcatchment 12S: CENTRAL PERF. DRAIN 2

Runoff = 0.73 cfs @ 12.08 hrs, Volume= 2,203 cf, Depth= 3.97"

	Area (sf)	CN	Description
*	6,658	6,658 63 Crushed Stone Surface	Crushed Stone Surface, HSG A
	6,658		100.00% Pervious Area

Prepare	CWW Substation 5-Parcel Proposed ConditType III 24-hr NRCC 100YR 24H Rainfall=8.38"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 15										
Tc (min)											
5.0					Direct Entry, Direct Entry						
	Summary for Subcatchment 13S: GIS BUILDINGS										
Runoff	=	4.29 cf	s@ 12.0	7 hrs, Volu	me= 14,855 cf, Depth= 8.14"						
				CS, Weigh infall=8.38"	ted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs						
A	rea (sf)	CN E	Description								
	21,900		Roofs, HSC								
	21,900	1	00.00% In	pervious A	rea						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
5.0					Direct Entry, Direct Entry						
	ę	Summa	ry for Su	ıbcatchm	ent 14S: GIS BLDGS PERF DRAIN						
Runoff	=	7.32 cf	s @ 12.1	2 hrs, Volu	me= 24,745 cf, Depth= 3.97"						
				CS, Weigh infall=8.38"	ted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs						
A	rea (sf)	CN E	Description								
*	74,801	63 C	Crushed St	one Surface	e, HSG A						
	74,801	1	00.00% Pe	ervious Are	a						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
2.3	50	0.0200	0.36		Sheet Flow, Sheet						
6.1	520	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps						
8.4	570	Total									
	Summary for Subcatchment 15S: CENTRAL PERF. DRAIN 1										
- "		F 00 7									

Runoff = 5.29 cfs @ 12.12 hrs, Volume= 17,893 cf, Depth= 3.97"

	Area (sf)	CN	Description		
*	54,087	63	Crushed Stone Surface, HSG A		
	54,087		100.00% Pervious Area		

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
	6.1	520	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc
	0.1	520	0.0200	1.41		Nearly Bare & Untilled Kv= 10.0 fps
_	8.4	570	Total			

Summary for Subcatchment 16S: N PERF. DRAIN

Runoff = 3.92 cfs @ 12.08 hrs, Volume= 11,771 cf, Depth= 3.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

_	A	rea (sf)	CN	Description						
*		35,381 202	63 98	Crushed St Roofs, HSC		e, HSG A				
_		35,583 35,381 202	63	Weighted A 99.43% Per 0.57% Impe	verage vious Area					
	Tc (min)	Length (feet)	Slop (ft/ft	,	Capacity (cfs)	Description				
	5.0					Direct Entry, Direct Entry				

Summary for Subcatchment 17S: SE PERF. DRAIN 1

Runoff = 5.38 cfs @ 12.09 hrs, Volume= 16,593 cf, Depth= 4.79"

	A	rea (sf)	CN [Description		
		8,310	98 F	Roofs, HSC	βA	
*		33,236	63 (Crushed St	one Surfac	e, HSG A
		41,546	70 \	Veighted A	verage	
		33,236	8	30.00% Pei	vious Area	
		8,310	2	20.00% Imp	pervious Are	ea
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
						Fallow n= 0.050 P2= 3.29"
	3.5	300	0.0200	1.41		Shallow Concentrated Flow, Shallow
						Nearly Bare & Untilled Kv= 10.0 fps
	0.1	75	0.0200	8.34	6.55	Pipe Channel, Perf. Pipe
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
_						n= 0.010 PVC, smooth interior
	5.9	425	Total			

Summary for Subcatchment 18S: SE PERF. DRAIN 2

Runoff = 3.30 cfs @ 12.07 hrs, Volume= 9,857 cf, Depth= 4.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

_	A	rea (sf)	CN	Description		
		4,255	98	Roofs, HSG	βA	
*		21,049	63	Crushed St	one Surfac	e, HSG A
		25,304 21,049 4,255	69 Slop	Weighted A 83.18% Per 16.82% Imp	vious Area pervious Ar	ea
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description
	5.0	(1901)	(1011	., ((010)	Direct Entry, Direct Entry

Summary for Subcatchment 20S: SR4

Runoff = 0.34 cfs @ 12.07 hrs, Volume= 1,184 cf, Depth= 8.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

A	rea (sf)	CN [CN Description					
	1,745	98 (8 Concrete Containment					
	1,745		100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry, Direct Entry			

Summary for Subcatchment 21S: XFMR 1

Runoff = 0.53 cfs @ 12.07 hrs, Volume= 1,827 cf, Depth= 8.14"

A	rea (sf)	CN E	Description				
	2,694	98 C	Concrete Containment				
	2,694	1	00.00% Im	pervious A	rea		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry, Direct Entry		

Summary for Subcatchment 22S: SR5

Runoff = 0.34 cfs @ 12.07 hrs, Volume= 1,184 cf, Depth= 8.14"

Are	ea (sf)	CN	Descripti	on		
	1,745	98	Concrete	Containme	ent	
	1,745		100.00%	Impervious	s Area	
Tc I (min)	Length (feet)	Slop (ft/f			ty Descriptio s)	n
5.0					Direct En	try, Direct Entry
			Sumr	nary for S	Subcatchm	ent 23S: XFMR 2
Runoff	=	0.53	cfs @ 12	2.07 hrs, V	olume=	1,827 cf, Depth= 8.14"
				=SCS, Wei Rainfall=8.3		ne Span= 0.00-72.00 hrs, dt= 0.01 hrs
Are	ea (sf)	CN	Descripti	on		
	2,694	98	Concrete	Containme	ent	
	2,694		100.00%	Impervious	s Area	
Tc I (min)	Length (feet)	Slop (ft/f			ty Descriptio s)	n
5.0					Direct En	try, Direct Entry
			Sumr	nary for S	Subcatchm	ent 24S: XFMR 3
Runoff	=	0.53	cfs @ 12	2.07 hrs, V	olume=	1,827 cf, Depth= 8.14"
				=SCS, Wei Rainfall=8.3		ne Span= 0.00-72.00 hrs, dt= 0.01 hrs
Are	ea (sf)	CN	Descripti	on		
	2,694	98	Concrete	Containme	ent	

	2,694	98 C	concrete Co	ontainment	
	2,694	1	00.00% Im	pervious A	rea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Direct Entry

Summary for Subcatchment 25S: SR6

Runoff = 0.34 cfs @ 12.07 hrs, Volume= 1,184 cf, Depth= 8.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

Ar	ea (sf)	CN [Description			
	1,745	98 (Concrete C	ontainment		
	1,745		100.00% In	npervious A	rea	
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description	
5.0					Direct Entry	/, Direct Entry
			Sumn	nary for S	Subcatchmo	ent 26S: SR1
Runoff	=	0.31 c	fs @ 12.0	7 hrs, Volu	ime=	1,085 cf, Depth= 8.14"
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"					
Ar	rea (sf)	CN [Description			
	1,600	98 (Concrete C	ontainment		
	1,600		100.00% In	npervious A	rea	
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description	
5.0					Direct Entry	/, Direct Entry
			Sumn	nary for S	Subcatchmo	ent 27S: SR2
Runoff	=	0.31 c	fs @ 12.0	7 hrs, Volu	ime=	1,085 cf, Depth= 8.14"
			hod, UH=S YR 24H Ra			Span= 0.00-72.00 hrs, dt= 0.01 hrs
Ar	ea (sf)	CN [Description			
	1,600	98 (Concrete C	ontainment		
	1,600		100.00% In	npervious A	rea	
Тс	Length	Slope	Velocity	Capacity	Description	

(feet)

(min)

5.0

(ft/ft)

(ft/sec)

(cfs)

Direct Entry, Direct Entry

Summary for Subcatchment 28S: SR3

Runoff = 0.31 cfs @ 12.07 hrs, Volume= 1,085 cf, Depth= 8.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 100YR 24H Rainfall=8.38"

A	rea (sf)	CN Description
	1,600	98 Concrete Containment
	1,600	100.00% Impervious Area
Tc (min)	Length (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)
5.0		Direct Entry, Direct Entry
	Su	nmary for Subcatchment 29S: STATCOM 1 TRANSFORMER
Runoff	=	0.35 cfs @ 12.07 hrs, Volume= 1,201 cf, Depth= 8.14"
		20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CC 100YR 24H Rainfall=8.38"
A	rea (sf)	CN Description
	1,770	98 Concrete Containment
	1,770	100.00% Impervious Area
Tc (min)	Length (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)
5.0		Direct Entry, Direct Entry
	Su	nmary for Subcatchment 30S: STATCOM 2 TRANSFORMER
Runoff	=	0.35 cfs @ 12.07 hrs, Volume= 1,201 cf, Depth= 8.14"
		-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CC 100YR 24H Rainfall=8.38"
A	rea (sf)	CN Description
	1,770	98 Concrete Containment
	1,770	100.00% Impervious Area

Description

Direct Entry, Direct Entry

(cfs)

Slope Velocity Capacity

(ft/sec)

Tc

(min)

5.0

Length

(feet)

(ft/ft)

Summary for Subcatchment 31S: STATCOM 3 TRANSFORMER

Runoff = 0.35 cfs @ 12.07 hrs, Volume= 1,201 cf, Depth= 8.14"

Area (sf) CN Description					
1,770 98 Concrete Containment					
1,770 100.00% Impervious Area					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
5.0 Direct Entry, Direct Entry					
Summary for Reach 1R: S SWALE PART 1					
Inflow Area = 21,901 sf, 0.00% Impervious, Inflow Depth = 3.16" for NRCC 100YR 24H event Inflow = 1.80 cfs @ 12.10 hrs, Volume= 5,769 cf					
Outflow = 1.70 cfs @ 12.12 hrs, Volume= 5,769 cf, Atten= 5%, Lag= 1.7 min					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.23 fps, Min. Travel Time= 2.3 min Avg. Velocity = 0.92 fps, Avg. Travel Time= 5.6 min					
Peak Storage= 236 cf @ 12.12 hrs Average Depth at Peak Storage= 0.50' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 38.49 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.56 cfs					
0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 309.0' Slope= 0.0137 '/' Inlet Invert= 174.13', Outlet Invert= 169.91'					

Summary for Reach 2R: S SWALE PART 2

Inflow Are	a =	59,308 sf, 0.00% Impervious, Inflow Depth = 2.13" for NRCC 100YR 24H event
Inflow	=	2.88 cfs @ 12.11 hrs, Volume= 10,513 cf
Outflow	=	2.81 cfs @ 12.13 hrs, Volume= 10,513 cf, Atten= 2%, Lag= 1.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.89 fps, Min. Travel Time= 1.5 min Avg. Velocity = 1.20 fps, Avg. Travel Time= 3.6 min

Peak Storage= 250 cf @ 12.13 hrs Average Depth at Peak Storage= 0.57' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 46.01 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 12.62 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 257.3' Slope= 0.0195 '/' Inlet Invert= 169.90', Outlet Invert= 164.88'

Summary for Reach 3R: S SWALE PART 3

 Inflow Area =
 87,839 sf,
 0.00% Impervious,
 Inflow Depth =
 2.17"
 for
 NRCC 100YR 24H event

 Inflow =
 4.20 cfs @
 12.14 hrs,
 Volume=
 15,897 cf

 Outflow =
 4.06 cfs @
 12.16 hrs,
 Volume=
 15,897 cf,
 Atten= 3%,
 Lag= 1.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.81 fps, Min. Travel Time= 1.9 min Avg. Velocity = 1.13 fps, Avg. Travel Time= 4.8 min

Peak Storage= 475 cf @ 12.16 hrs Average Depth at Peak Storage= 0.69' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 39.24 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.77 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 329.0' Slope= 0.0142 '/' Inlet Invert= 164.80', Outlet Invert= 160.13'

Summary for Reach 4R: S SWALE PART 4

Inflow Area = 125,461 sf, 0.00% Impervious, Inflow Depth = 2.96" for NRCC 100YR 24H event Inflow = 7.94 cfs @ 12.13 hrs, Volume= 30,904 cf Outflow = 7.66 cfs @ 12.16 hrs, Volume= 30,904 cf, Atten= 4%, Lag= 1.9 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Max. Velocity= 2.57 fps, Min. Travel Time= 2.4 min Avg. Velocity = 0.91 fps, Avg. Travel Time= 6.8 min

Peak Storage= 1,094 cf @ 12.16 hrs Average Depth at Peak Storage= 1.00' Defined Flood Depth= 2.00' Flow Area= 9.5 sf, Capacity= 31.59 cfs Bank-Full Depth= 1.10' Flow Area= 3.6 sf, Capacity= 9.98 cfs

0.00' x 1.10' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.60' Length= 367.8' Slope= 0.0073 '/' Inlet Invert= 157.70', Outlet Invert= 155.00'

Summary for Reach 5R: DRAINAGE CHANNEL

 Inflow Area =
 497,238 sf, 11.68% Impervious, Inflow Depth = 3.98" for NRCC 100YR 24H event

 Inflow =
 47.83 cfs @ 12.12 hrs, Volume=
 165,100 cf

 Outflow =
 47.76 cfs @ 12.12 hrs, Volume=
 165,100 cf, Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 5.74 fps, Min. Travel Time= 0.5 min Avg. Velocity = 1.20 fps, Avg. Travel Time= 2.4 min

Peak Storage= 1,456 cf @ 12.12 hrs Average Depth at Peak Storage= 0.42' Defined Flood Depth= 1.00' Flow Area= 20.0 sf, Capacity= 198.63 cfs Bank-Full Depth= 1.00' Flow Area= 20.0 sf, Capacity= 198.63 cfs

20.00' x 1.00' deep channel, n= 0.078 Riprap, 12-inch Length= 175.0' Slope= 0.3086 '/' Inlet Invert= 145.00', Outlet Invert= 91.00'

Summary for Reach 11R: N SWALE PART 1

Inflow Area = 15.778 sf. 0.00% Impervious. Inflow Depth = 5.98" for NRCC 100YR 24H event Inflow 2.48 cfs @ 12.09 hrs. Volume= 7.865 cf = 2.29 cfs @ 12.12 hrs, Volume= Outflow = 7,865 cf, Atten= 7%, Lag= 2.0 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.51 fps, Min. Travel Time= 3.0 min Avg. Velocity = 0.93 fps, Avg. Travel Time= 8.0 min Peak Storage= 408 cf @ 12.12 hrs Average Depth at Peak Storage= 0.55' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 40.78 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 11.19 cfs 0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 447.0' Slope= 0.0153 '/' Inlet Invert= 174.03', Outlet Invert= 167.18'

Summary for Reach 12R: N SWALE PART 2

 Inflow Area =
 29,879 sf,
 0.00% Impervious,
 Inflow Depth =
 6.15"
 for
 NRCC 100YR 24H event

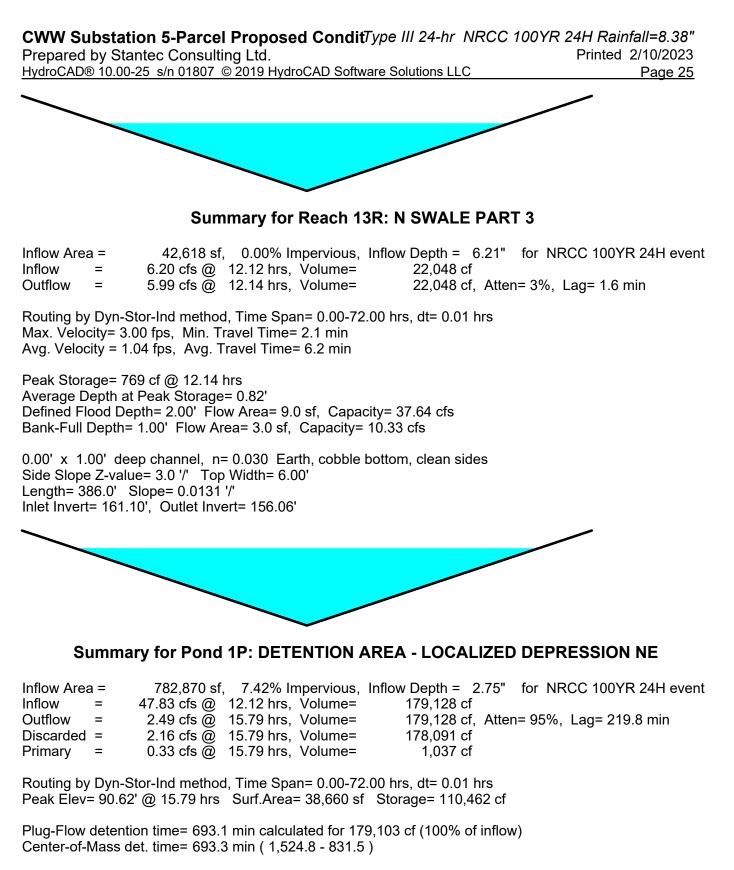
 Inflow =
 4.52 cfs @
 12.10 hrs,
 Volume=
 15,317 cf

 Outflow =
 4.29 cfs @
 12.13 hrs,
 Volume=
 15,317 cf,
 Atten= 5%,
 Lag= 1.8 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.82 fps, Min. Travel Time= 2.6 min Avg. Velocity = 0.99 fps, Avg. Travel Time= 7.2 min

Peak Storage= 656 cf @ 12.13 hrs Average Depth at Peak Storage= 0.71' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 38.60 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.59 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 431.0' Slope= 0.0137 '/' Inlet Invert= 167.10', Outlet Invert= 161.18'



Volume	Invert	Avail.Storage	Storage Description
#1	83.00'	125,742 cf	Custom Stage Data (Irregular)Listed below (Recalc)

CWW Substation 5-Parcel Proposed Condit*Type III 24-hr NRCC 100YR 24H Rainfall=8.38"* Prepared by Stantec Consulting Ltd. Printed 2/10/2023

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Page 26

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
83.00	827	135.0	0	0	827
84.00	2,756	217.0	1,698	1,698	3,131
85.00	5,016	298.0	3,830	5,528	6,460
86.00	8,057	383.0	6,477	12,004	11,079
87.00	12,347	488.0	10,126	22,130	18,370
88.00	17,968	589.0	15,070	37,200	27,043
89.00	25,096	690.0	21,433	58,633	37,342
90.00	33,858	865.0	29,368	88,001	59,011
91.00	41,761	936.1	37,740	125,742	69,242

Device	Routing	Invert	Outlet Devices
#1	Discarded	83.00'	2.410 in/hr Exfiltration over Surface area
#2	Primary	90.60'	50.0' long x 4.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66
			2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32

Discarded OutFlow Max=2.16 cfs @ 15.79 hrs HW=90.62' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 2.16 cfs)

Primary OutFlow Max=0.33 cfs @ 15.79 hrs HW=90.62' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 0.33 cfs @ 0.34 fps)

Summary for Pond 2P: SEDIMENT FOREBAY

Inflow Area =	168,079 sf, 0.00% Impervious,	Inflow Depth = 3.78" for NRCC 100YR 24H event
Inflow =	13.61 cfs @ 12.15 hrs, Volume=	52,952 cf
Outflow =	13.49 cfs @ 12.17 hrs, Volume=	52,953 cf, Atten= 1%, Lag= 0.9 min
Discarded =	0.14 cfs @ 12.17 hrs, Volume=	16,828 cf
Primary =	13.35 cfs @ 12.17 hrs, Volume=	36,124 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 150.40' @ 12.17 hrs Surf.Area= 2,464 sf Storage= 9,809 cf Flood Elev= 151.00' Surf.Area= 2,531 sf Storage= 11,301 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 227.3 min (1,061.9 - 834.6)

Volume	Invert	Avail.Sto	rage Storage	Description		
#1	146.00'	22,34	18 cf Custom	Stage Data (Con	ic) Listed below (Re	calc)
Elevatio (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
146.0 155.0	-	2,000 3,000	0 22,348	0 22,348	2,000 3,876	
Device	Routing	Invert	Outlet Devices	6		
#1 #2	Discarded Primary	146.00' 150.00'	-	cfiltration over Su 2.0' breadth Broad	rface area d-Crested Rectang	gular Weir

> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.14 cfs @ 12.17 hrs HW=150.40' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.14 cfs)

Primary OutFlow Max=13.34 cfs @ 12.17 hrs HW=150.40' TW=145.39' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 13.34 cfs @ 1.66 fps)

Summary for Pond 3P: DMHs w/ Vortex Unit

 Inflow Area =
 329,159 sf, 17.65% Impervious, Inflow Depth = 4.70" for NRCC 100YR 24H event

 Inflow =
 38.54 cfs @ 12.09 hrs, Volume=
 128,976 cf

 Outflow =
 38.54 cfs @ 12.09 hrs, Volume=
 128,976 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 38.54 cfs @ 12.09 hrs, Volume=
 128,976 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 150.76' @ 12.09 hrs Flood Elev= 151.05'

DeviceRoutingInvertOutlet Devices#1Primary148.00'36.0" Round Culvert L= 25.0' Ke= 0.500
Inlet / Outlet Invert= 148.00' / 145.00' S= 0.1200 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf

Primary OutFlow Max=38.53 cfs @ 12.09 hrs HW=150.76' TW=145.38' (Dynamic Tailwater) -1=Culvert (Inlet Controls 38.53 cfs @ 5.66 fps)

Summary for Pond 4P: DMH

 Inflow Area =
 329,159 sf, 17.65% Impervious, Inflow Depth = 4.70" for NRCC 100YR 24H event

 Inflow =
 38.54 cfs @ 12.09 hrs, Volume=
 128,976 cf

 Outflow =
 38.54 cfs @ 12.09 hrs, Volume=
 128,976 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 38.54 cfs @ 12.09 hrs, Volume=
 128,976 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.06' @ 12.09 hrs Flood Elev= 158.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	150.30'	36.0" Round Culvert L= 36.0' Ke= 0.500 Inlet / Outlet Invert= 150.30' / 148.00' S= 0.0639 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf

Summary for Pond 5P: DMH

Inflow Area = 216,616 sf, 16.26% Impervious, Inflow Depth = 4.65" for NRCC 100YR 24H event Inflow 24.21 cfs @ 12.10 hrs, Volume= 83.898 cf = 24.21 cfs @ 12.10 hrs, Volume= Outflow = 83,898 cf, Atten= 0%, Lag= 0.0 min 24.21 cfs @ 12.10 hrs, Volume= Primary = 83.898 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 158.96' @ 12.10 hrs

Flood Elev= 161.58'

Device	Routing	Invert	Outlet Devices
#1	Primary	155.40'	24.0" Round Culvert L= 263.0' Ke= 0.500 Inlet / Outlet Invert= 155.40' / 150.50' S= 0.0186 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=24.18 cfs @ 12.10 hrs HW=158.96' TW=153.05' (Dynamic Tailwater) -1=Culvert (Inlet Controls 24.18 cfs @ 7.70 fps)

Summary for Pond 6P: OIL/WATER SEPARATOR

Inflow Area	a =	13,317 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow	=	2.61 cfs @ 12.07 hrs, Volume= 9,033 cf
Outflow	=	2.61 cfs @ 12.07 hrs, Volume= 9,033 cf, Atten= 0%, Lag= 0.0 min
Primary	=	2.61 cfs @ 12.07 hrs, Volume= 9,033 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 158.98' @ 12.11 hrs Flood Elev= 162.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	155.90'	24.0" Round Culvert L= 27.0' Ke= 0.500 Inlet / Outlet Invert= 155.90' / 155.50' S= 0.0148 '/' Cc= 0.900 n= 0.012, Flow Area= 3.14 sf

Primary OutFlow Max=0.00 cfs @ 12.07 hrs HW=158.56' TW=158.72' (Dynamic Tailwater)

Summary for Pond 7P: DMH

Inflow Area =13,317 sf,100.00% Impervious, Inflow Depth =8.14"for NRCC 100YR 24H eventInflow =2.61 cfs @12.07 hrs, Volume=9,033 cfOutflow =2.61 cfs @12.07 hrs, Volume=9,033 cf, Atten= 0%, Lag= 0.0 minPrimary =2.61 cfs @12.07 hrs, Volume=9,033 cfRouting by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs10.01 hrs

Peak Elev= 159.34' @ 12.11 hrs

Flood Elev= 162.77'

Device	Routing	Invert	Outlet Devices
#1	Primary	156.40'	12.0" Round Culvert L= 40.0' Ke= 0.500

> Inlet / Outlet Invert= 156.40' / 156.00' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.88 cfs @ 12.07 hrs HW=158.80' TW=158.56' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.88 cfs @ 2.39 fps)

Summary for Pond 8P: DMH

Inflow Area :	=	8,878 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow =	=	1.74 cfs @ 12.07 hrs, Volume= 6,022 cf
Outflow =	=	1.74 cfs @ 12.07 hrs, Volume= 6,022 cf, Atten= 0%, Lag= 0.0 min
Primary =	=	1.74 cfs @ 12.07 hrs, Volume= 6,022 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.60' @ 12.12 hrs Flood Elev= 164.88'

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 157.50'
 12.0" Round Culvert L= 154.0' Ke= 0.500 Inlet / Outlet Invert= 157.50' / 156.50' S= 0.0065 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.99 cfs @ 12.07 hrs HW=158.92' TW=158.80' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.99 cfs @ 1.26 fps)

Summary for Pond 9P: DMH

Inflow Are	a =	4,439 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow	=	0.87 cfs @ 12.07 hrs, Volume= 3,011 cf
Outflow	=	0.87 cfs @ 12.07 hrs, Volume= 3,011 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.87 cfs @ 12.07 hrs, Volume= 3,011 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.91' @ 12.12 hrs Flood Elev= 166.83'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.30'	12.0" Round Culvert L= 141.0' Ke= 0.500 Inlet / Outlet Invert= 159.30' / 158.00' S= 0.0092 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.77 cfs @ 12.07 hrs HW=159.78' TW=158.92' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.77 cfs @ 3.06 fps)

Summary for Pond 10P: 24" Petro-Barrier

Inflow Are	ea =	1,745 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow	=	0.34 cfs @ 12.07 hrs, Volume= 1,184 cf
Outflow	=	0.34 cfs @ 12.07 hrs, Volume= 1,184 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.34 cfs @ 12.07 hrs, Volume= 1,184 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.81' @ 12.07 hrs Flood Elev= 169.43'

Device	Routing	Invert	Outlet Devices
#1	Primary	162.43'	6.0" Round Culvert L= 30.0' Ke= 0.500 Inlet / Outlet Invert= 162.43' / 162.00' S= 0.0143 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.34 cfs @ 12.07 hrs HW=162.81' TW=159.78' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.34 cfs @ 2.11 fps)

Summary for Pond 11P: 24" Petro-Barrier

Inflow Are	a =	2,694 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow	=	0.53 cfs @ 12.07 hrs, Volume= 1,827 cf
Outflow	=	0.53 cfs @ 12.07 hrs, Volume= 1,827 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.53 cfs @ 12.07 hrs, Volume= 1,827 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 160.18' @ 12.07 hrs Flood Elev= 168.62'

DeviceRoutingInvertOutlet Devices#1Primary159.62'6.0" Round Culvert L= 18.0' Ke= 0.500
Inlet / Outlet Invert= 159.62' / 159.40' S= 0.0122 '/' Cc= 0.900
n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.53 cfs @ 12.07 hrs HW=160.18' TW=159.78' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.53 cfs @ 2.68 fps)

Summary for Pond 12P: 24" Petro-Barrier

Inflow Are	a =	1,745 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow	=	0.34 cfs @ 12.07 hrs, Volume= 1,184 cf
Outflow	=	0.34 cfs @ 12.07 hrs, Volume= 1,184 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.34 cfs @ 12.07 hrs, Volume= 1,184 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 161.08' @ 12.07 hrs Flood Elev= 167.70'

Device	Routing	Invert	Outlet Devices	
#1	Primary	160.70'	6.0" Round Culvert L= 32.0' Ke= 0.500 Inlet / Outlet Invert= 160.70' / 160.40' S= 0.0094 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf	

Primary OutFlow Max=0.34 cfs @ 12.07 hrs HW=161.08' TW=158.92' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.34 cfs @ 2.11 fps)

Summary for Pond 13P: 24" Petro-Barrier

Inflow Area = 2,694 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event Inflow 0.53 cfs @ 12.07 hrs. Volume= 1.827 cf = 0.53 cfs @ 12.07 hrs, Volume= Outflow = 1,827 cf, Atten= 0%, Lag= 0.0 min 0.53 cfs @ 12.07 hrs, Volume= Primary = 1.827 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.81' @ 12.12 hrs Flood Elev= 166.83' Device Routing Invert Outlet Devices

#1	Primary	157.83'	6.0" Round Culvert L= 20.0' Ke= 0.500
	-		Inlet / Outlet Invert= 157.83' / 157.60' S= 0.0115 '/' Cc= 0.900
			n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.18 cfs @ 12.07 hrs HW=158.96' TW=158.92' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.18 cfs @ 0.94 fps)

Summary for Pond 14P: 24" Petro-Barrier

Inflow Are	a =	2,694 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow	=	0.53 cfs @ 12.07 hrs, Volume= 1,827 cf
Outflow	=	0.53 cfs @ 12.07 hrs, Volume= 1,827 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.53 cfs @ 12.07 hrs, Volume= 1,827 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.55' @ 12.12 hrs Flood Elev= 165.88'

Device	Routing	Invert	Outlet Devices	
#1	Primary	156.88'	6.0" Round Culvert L= 21.0' Ke= 0.500 Inlet / Outlet Invert= 156.88' / 156.60' S= 0.0133 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf	

Primary OutFlow Max=0.22 cfs @ 12.07 hrs HW=158.86' TW=158.80' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 0.22 cfs @ 1.14 fps)

Summary for Pond 15P: 24" Petro-Barrier

 Inflow Area =
 1,745 sf,100.00% Impervious, Inflow Depth =
 8.14" for NRCC 100YR 24H event

 Inflow =
 0.34 cfs @
 12.07 hrs, Volume=
 1,184 cf

 Outflow =
 0.34 cfs @
 12.07 hrs, Volume=
 1,184 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.34 cfs @
 12.07 hrs, Volume=
 1,184 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.44' @ 12.12 hrs Flood Elev= 164.66'

Device	Routing	Invert	Outlet Devices
#1	Primary	157.66'	6.0" Round Culvert L= 34.0' Ke= 0.500

Inlet / Outlet Invert= 157.66' / 157.40' S= 0.0076 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.00 cfs @ 12.07 hrs HW=158.69' TW=158.80' (Dynamic Tailwater)

Summary for Pond 16P: DMH

Inflow Area	=	52,511 sf, 0.00% Impervious, Inflow Depth = 3.97" for NRCC 100YR 24H event
Inflow :	=	5.60 cfs @ 12.09 hrs, Volume= 17,371 cf
Outflow :	=	5.60 cfs @ 12.09 hrs, Volume= 17,371 cf, Atten= 0%, Lag= 0.0 min
Primary :	=	5.60 cfs @ 12.09 hrs, Volume= 17,371 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 161.70' @ 12.09 hrs Flood Elev= 162.19'

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 159.00'
 12.0" Round Culvert L= 17.0' Ke= 0.500 Inlet / Outlet Invert= 159.00' / 158.50' S= 0.0294 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=5.60 cfs @ 12.09 hrs HW=161.69' TW=158.93' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 5.60 cfs @ 7.13 fps)

Summary for Pond 17P: DMH

Inflow Are	ea =	150,788 sf, 14.52% Impervious, Inflow Depth = 4.58" for NRCC 100YR 24H event
Inflow	=	16.29 cfs @ 12.11 hrs, Volume= 57,493 cf
Outflow	=	16.29 cfs @ 12.11 hrs, Volume= 57,493 cf, Atten= 0%, Lag= 0.0 min
Primary	=	16.29 cfs @ 12.11 hrs, Volume= 57,493 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 161.56' @ 12.11 hrs Flood Elev= 163.59'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.40'	24.0" Round Culvert L= 88.2' Ke= 0.500 Inlet / Outlet Invert= 159.40' / 155.50' S= 0.0442 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=16.27 cfs @ 12.11 hrs HW=161.56' TW=158.93' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 16.27 cfs @ 5.18 fps)

Summary for Pond 18P: DMH

Inflow Are	ea =	128,888 sf, 0.00% Impervious, Inflow Depth = 3.97" for NRCC 100YR 24H event
Inflow	=	12.61 cfs @ 12.12 hrs, Volume= 42,638 cf
Outflow	=	12.61 cfs @ 12.12 hrs, Volume= 42,638 cf, Atten= 0%, Lag= 0.0 min
Primary	=	12.61 cfs @ 12.12 hrs, Volume= 42,638 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 164.45' @ 12.12 hrs Flood Elev= 165.39'

Device	Routing	Invert	Outlet Devices	
#1	Primary	161.50'	18.0" Round Culvert L= 92.0' Ke= 0.500	
			Inlet / Outlet Invert= 161.50' / 159.50' S= 0.0217 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf	

Primary OutFlow Max=12.60 cfs @ 12.12 hrs HW=164.44' TW=161.53' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 12.60 cfs @ 7.13 fps)

Summary for Pond 20P: DMH

Inflow Are	ea =	102,433 sf, 12.46% Impervious, Inflow Depth = 4.48" for NRCC 100YR 24H event
Inflow	=	12.58 cfs @ 12.08 hrs, Volume= 38,221 cf
Outflow	=	12.58 cfs @ 12.08 hrs, Volume= 38,221 cf, Atten= 0%, Lag= 0.0 min
Primary	=	12.58 cfs @ 12.08 hrs, Volume= 38,221 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.87' @ 12.09 hrs Flood Elev= 158.03'

DeviceRoutingInvertOutlet Devices#1Primary152.00'24.0" Round Culvert L= 56.0' Ke= 0.500
Inlet / Outlet Invert= 152.00' / 150.40' S= 0.0286 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=12.26 cfs @ 12.08 hrs HW=153.86' TW=153.05' (Dynamic Tailwater) -1=Culvert (Outlet Controls 12.26 cfs @ 5.25 fps)

Summary for Pond 21P: DMH

Inflow Area =		25,304 sf, 16.82% Impervious, Inflow Depth = 4.67" for NRCC 100YR 24H event
Inflow	=	3.30 cfs @ 12.07 hrs, Volume= 9,857 cf
Outflow	=	3.30 cfs @ 12.07 hrs, Volume= 9,857 cf, Atten= 0%, Lag= 0.0 min
Primary	=	3.30 cfs @ 12.07 hrs, Volume= 9,857 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 156.20' @ 12.07 hrs Flood Elev= 157.94'

Device	Routing	Invert	Outlet Devices
#1	Primary	154.94'	12.0" Round Culvert L= 62.0' Ke= 0.500 Inlet / Outlet Invert= 154.94' / 152.40' S= 0.0410 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=3.29 cfs @ 12.07 hrs HW=156.20' TW=153.83' (Dynamic Tailwater) -1=Culvert (Inlet Controls 3.29 cfs @ 4.19 fps)

Summary for Pond 22P: DMH

Inflow Area = 77.129 sf. 11.04% Impervious. Inflow Depth = 4.41° for NRCC 100YR 24H event Inflow 9.29 cfs @ 12.08 hrs. Volume= 28.364 cf = 9.29 cfs @ 12.08 hrs, Volume= Outflow = 28,364 cf. Atten= 0%, Lag= 0.0 min 9.29 cfs @ 12.08 hrs, Volume= Primary = 28,364 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 157.44' @ 12.08 hrs Flood Elev= 158.66' Device Routing Invert Outlet Devices #1 155.50' 18.0" Round Culvert L= 56.0' Ke= 0.500 Primary Inlet / Outlet Invert= 155.50' / 155.00' S= 0.0089 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=9.27 cfs @ 12.08 hrs HW=157.44' TW=153.86' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 9.27 cfs @ 5.25 fps)

Summary for Pond 23P: OIL/WATER SEPARATOR

Inflow Area =		10,110 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow	=	1.98 cfs @ 12.07 hrs, Volume= 6,858 cf
Outflow	=	2.01 cfs @ 12.07 hrs, Volume= 6,858 cf, Atten= 0%, Lag= 0.0 min
Primary	=	2.01 cfs @ 12.07 hrs, Volume= 6,858 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.57' @ 12.09 hrs Flood Elev= 159.18'

Device	Routing	Invert	Outlet Devices
#1	Primary	141.80'	12.0" Round Culvert L= 182.0' Ke= 0.500 Inlet / Outlet Invert= 141.80' / 140.10' S= 0.0093 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.86 cfs @ 12.07 hrs HW=153.49' TW=153.00' (Dynamic Tailwater) -1=Culvert (Outlet Controls 1.86 cfs @ 2.37 fps)

Summary for Pond 24P: DMH

 Inflow Area =
 10,110 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event

 Inflow =
 1.98 cfs @ 12.07 hrs, Volume=
 6,858 cf

 Outflow =
 1.98 cfs @ 12.07 hrs, Volume=
 6,858 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 1.98 cfs @ 12.07 hrs, Volume=
 6,858 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.81' @ 12.10 hrs Flood Elev= 159.61'

Device	Routing	Invert	Outlet Devices
#1	Primary	150.20'	12.0" Round Culvert L= 1.0' Ke= 0.500

Inlet / Outlet Invert= 150.20' / 150.10' S= 0.1000 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.40 cfs @ 12.07 hrs HW=153.63' TW=153.49' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.40 cfs @ 1.79 fps)

Summary for Pond 25P: 24" Petro-Barrier

Inflow Area =		1,770 sf,100.00% Impervious,		Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow =	0.3	5 cfs @	12.07 hrs, Volume=	1,201 cf
Outflow =	0.3	5 cfs @	12.07 hrs, Volume=	1,201 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.3	5 cfs @	12.07 hrs, Volume=	1,201 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.98' @ 12.11 hrs Flood Elev= 159.89'

DeviceRoutingInvertOutlet Devices#1Primary150.89'6.0" Round Culvert L= 58.0' Ke= 0.500
Inlet / Outlet Invert= 150.89' / 150.30' S= 0.0102 '/' Cc= 0.900
n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.22 cfs @ 12.07 hrs HW=153.71' TW=153.63' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.22 cfs @ 1.11 fps)

Summary for Pond 26P: 24" Petro-Barrier

Inflow Area =		1,770 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow	=	0.35 cfs @ 12.07 hrs, Volume= 1,201 cf
Outflow	=	0.35 cfs @ 12.07 hrs, Volume= 1,201 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.35 cfs @ 12.07 hrs, Volume= 1,201 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.92' @ 12.11 hrs Flood Elev= 161.98'

Device	Routing	Invert	Outlet Devices
#1	Primary	152.98'	6.0" Round Culvert L= 17.0' Ke= 0.500 Inlet / Outlet Invert= 152.98' / 152.00' S= 0.0576 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.10 cfs @ 12.07 hrs HW=153.64' TW=153.63' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.10 cfs @ 0.51 fps)

Summary for Pond 27P: DMH

Inflow Are	ea =	6,570 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow	=	1.29 cfs @ 12.07 hrs, Volume= 4,457 cf
Outflow	=	1.29 cfs @ 12.07 hrs, Volume= 4,457 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.29 cfs @ 12.07 hrs, Volume=

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 154.61' @ 12.09 hrs Flood Elev= 162.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	153.90'	12.0" Round Culvert L= 224.0' Ke= 0.500 Inlet / Outlet Invert= 153.90' / 152.00' S= 0.0085 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.21 cfs @ 12.07 hrs HW=154.57' TW=153.63' (Dynamic Tailwater) -1=Culvert (Outlet Controls 1.21 cfs @ 3.06 fps)

Summary for Pond 28P: DMH

Inflow Are	a =	6,570 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow	=	1.29 cfs @ 12.07 hrs, Volume= 4,457 cf
Outflow	=	1.29 cfs @ 12.07 hrs, Volume= 4,457 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.29 cfs @ 12.07 hrs, Volume= 4,457 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 155.04' @ 12.08 hrs Flood Elev= 163.41'

DeviceRoutingInvertOutlet Devices#1Primary154.40'**12.0" Round Culvert** L= 50.0' Ke= 0.500
Inlet / Outlet Invert= 154.40' / 154.00' S= 0.0080 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.24 cfs @ 12.07 hrs HW=155.04' TW=154.57' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 1.24 cfs @ 3.35 fps)

Summary for Pond 29P: 24" Petro-Barrier

 Inflow Area =
 1,770 sf,100.00% Impervious, Inflow Depth =
 8.14" for NRCC 100YR 24H event

 Inflow =
 0.35 cfs @
 12.07 hrs, Volume=
 1,201 cf

 Outflow =
 0.35 cfs @
 12.07 hrs, Volume=
 1,201 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.35 cfs @
 12.07 hrs, Volume=
 1,201 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 155.34' @ 12.07 hrs Flood Elev= 163.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	154.95'	6.0" Round Culvert L= 7.0' Ke= 0.500 Inlet / Outlet Invert= 154.95' / 154.50' S= 0.0643 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.35 cfs @ 12.07 hrs HW=155.34' TW=155.04' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.35 cfs @ 2.12 fps)

Summary for Pond 30P: DMH

Inflow Area = 4,800 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event Inflow 0.94 cfs @ 12.07 hrs. Volume= 3.256 cf = 0.94 cfs @ 12.07 hrs, Volume= Outflow = 3,256 cf, Atten= 0%, Lag= 0.0 min 0.94 cfs @ 12.07 hrs, Volume= Primary = 3.256 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 160.90' @ 12.07 hrs Flood Elev= 165.34' Device Routing Invert Outlet Devices #1 160.40' 12.0" Round Culvert L= 98.0' Ke= 0.500 Primary Inlet / Outlet Invert= 160.40' / 159.50' S= 0.0092 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.94 cfs @ 12.07 hrs HW=160.90' TW=155.04' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.94 cfs @ 2.40 fps)

Summary for Pond 31P: DMH

 Inflow Area =
 4,800 sf,100.00% Impervious, Inflow Depth =
 8.14" for NRCC 100YR 24H event

 Inflow =
 0.94 cfs @
 12.07 hrs, Volume=
 3,256 cf

 Outflow =
 0.94 cfs @
 12.07 hrs, Volume=
 3,256 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.94 cfs @
 12.07 hrs, Volume=
 3,256 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.41' @ 12.07 hrs Flood Elev= 168.53'

Device	Routing	Invert	Outlet Devices
#1	Primary	161.90'	12.0" Round Culvert L= 232.0' Ke= 0.500
			Inlet / Outlet Invert= 161.90' / 160.50' S= 0.0060 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.94 cfs @ 12.07 hrs HW=162.41' TW=160.90' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.94 cfs @ 3.43 fps)

Summary for Pond 32P: 24" Petro-Barrier

 Inflow Area =
 1,600 sf,100.00% Impervious, Inflow Depth =
 8.14" for NRCC 100YR 24H event

 Inflow =
 0.31 cfs @
 12.07 hrs, Volume=
 1,085 cf

 Outflow =
 0.31 cfs @
 12.07 hrs, Volume=
 1,085 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.31 cfs @
 12.07 hrs, Volume=
 1,085 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.80' @ 12.07 hrs Flood Elev= 170.69'

Device	Routing	Invert	Outlet Devices
#1	Primary	162.44'	6.0" Round Culvert L= 10.0' Ke= 0.500

Inlet / Outlet Invert= 162.44' / 162.20' S= 0.0240 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.31 cfs @ 12.07 hrs HW=162.80' TW=162.41' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.31 cfs @ 2.05 fps)

Summary for Pond 33P: DMH

Inflow Area =	3,200 sf,100.00% Impervious,	Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow =	0.63 cfs @ 12.07 hrs, Volume=	2,171 cf
Outflow =	0.63 cfs @ 12.07 hrs, Volume=	2,171 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.63 cfs @ 12.07 hrs, Volume=	2,171 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 163.60' @ 12.07 hrs Flood Elev= 170.43'

DeviceRoutingInvertOutlet Devices#1Primary163.20'**12.0" Round Culvert** L= 159.0' Ke= 0.500
Inlet / Outlet Invert= 163.20' / 162.00' S= 0.0075 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.63 cfs @ 12.07 hrs HW=163.60' TW=162.41' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.63 cfs @ 2.15 fps)

Summary for Pond 34P: 24" Petro-Barrier

Inflow Area =		1,600 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow :	=	0.31 cfs @ 12.07 hrs, Volume= 1,085 cf
Outflow =	=	0.31 cfs @ 12.07 hrs, Volume= 1,085 cf, Atten= 0%, Lag= 0.0 min
Primary :	=	0.31 cfs @ 12.07 hrs, Volume= 1,085 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 163.81' @ 12.07 hrs Flood Elev= 171.68'

Device	Routing	Invert	Outlet Devices
#1	Primary	163.43'	6.0" Round Culvert L= 8.0' Ke= 0.500 Inlet / Outlet Invert= 163.43' / 163.30' S= 0.0162 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.31 cfs @ 12.07 hrs HW=163.81' TW=163.60' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.31 cfs @ 2.71 fps)

Summary for Pond 35P: 24" Petro-Barrier

Inflow Are	ea =	1,600 sf,100.00% Impervious, Inflow Depth = 8.14" for NRCC 100YR 24H event
Inflow	=	0.31 cfs @ 12.07 hrs, Volume= 1,085 cf
Outflow	=	0.31 cfs @ 12.07 hrs, Volume= 1,085 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.31 cfs @ 12.07 hrs, Volume= 1,085 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 164.75' @ 12.07 hrs Flood Elev= 172.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	164.39'	6.0" Round Culvert L= 38.0' Ke= 0.500 Inlet / Outlet Invert= 164.39' / 164.00' S= 0.0103 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.31 cfs @ 12.07 hrs HW=164.75' TW=163.60' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.31 cfs @ 2.05 fps)

Summary for Pond 40P: DMH

Inflow Area	=	105,170 sf, 0.00% Impervious, Inflow Depth = 2.76" for NRCC 100YR 24H event
Inflow	=	6.02 cfs @ 12.13 hrs, Volume= 24,192 cf
Outflow	=	6.02 cfs @ 12.13 hrs, Volume= 24,192 cf, Atten= 0%, Lag= 0.0 min
Primary	=	6.02 cfs @ 12.13 hrs, Volume= 24,192 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.13' @ 12.14 hrs Flood Elev= 160.03'

Device	Routing	Invert	Outlet Devices
#1	Primary		24.0" W x 12.0" H Box Culvert L= 10.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 158.00' / 157.70' S= 0.0300 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 2.00 sf

Primary OutFlow Max=5.96 cfs @ 12.13 hrs HW=159.12' TW=158.68' (Dynamic Tailwater) -1=Culvert (Inlet Controls 5.96 cfs @ 2.98 fps)

Summary for Pond 41P: CULVERT

Inflow Area	=	87,839 sf, 0.00% Impervious, Inflow Depth = 2.17" for NRCC 100YR 24H event
Inflow	=	4.06 cfs @ 12.16 hrs, Volume= 15,897 cf
Outflow	=	4.06 cfs @ 12.16 hrs, Volume= 15,897 cf, Atten= 0%, Lag= 0.0 min
Primary	=	4.06 cfs @ 12.16 hrs, Volume= 15,897 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.74' @ 12.16 hrs Flood Elev= 161.08'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.00'	24.0" W x 12.0" H Box Culvert L= 55.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 159.00' / 158.00' S= 0.0182 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 2.00 sf

Primary OutFlow Max=4.07 cfs @ 12.16 hrs HW=159.74' TW=159.11' (Dynamic Tailwater)

Summary for Link 1L: OVERFLOW

 Inflow Area =
 782,870 sf, 7.42% Impervious, Inflow Depth = 0.02" for NRCC 100YR 24H event

 Inflow =
 0.33 cfs @ 15.79 hrs, Volume=
 1,037 cf

 Primary =
 0.33 cfs @ 15.79 hrs, Volume=
 1,037 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Summary for Link 10L: OFF-SITE

Inflow Are	a =	258,650 sf, 0.00% Impervious, Inflow Depth = 0.67" for NRCC 100YR 24H event
Inflow	=	1.69 cfs @ 12.31 hrs, Volume= 14,486 cf
Primary	=	1.69 cfs @ 12.31 hrs, Volume= 14,486 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment1S: AREA OUTSIDE	Runoff Area=258,650 sf 0.00% Impervious Runoff Depth=0.02" Tc=5.0 min CN=32 Runoff=0.01 cfs 332 cf
Subcatchment2S: AREA OUTSIDE	Runoff Area=285,632 sf 0.00% Impervious Runoff Depth=0.01" Flow Length=491' Tc=12.3 min CN=31 Runoff=0.01 cfs 151 cf
Subcatchment 3S: S SWALE 1	Runoff Area=21,901 sf 0.00% Impervious Runoff Depth=0.96"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=56 Runoff=0.45 cfs 1,743 cf
Subcatchment4S: S SWALE 2	Runoff Area=37,407 sf 0.00% Impervious Runoff Depth=0.23" Tc=5.0 min CN=41 Runoff=0.05 cfs 727 cf
Subcatchment 5S: S SWALE 3	Runoff Area=28,531 sf 0.00% Impervious Runoff Depth=0.53"
Flow Length=644'	Slope=0.0200 '/' Tc=9.3 min CN=48 Runoff=0.17 cfs 1,250 cf
Subcatchment 6S: ACCESS RAMP	Runoff Area=17,331 sf 0.00% Impervious Runoff Depth=2.57" Tc=5.0 min CN=78 Runoff=1.24 cfs 3,709 cf
Subcatchment7S: S SWALE 4	Runoff Area=20,291 sf 0.00% Impervious Runoff Depth=1.40"
Flow Length=644'	Slope=0.0200 '/' Tc=9.3 min CN=63 Runoff=0.63 cfs 2,371 cf
Subcatchment 8S: N SWALE 1	Runoff Area=15,778 sf 0.00% Impervious Runoff Depth=2.75"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=80 Runoff=1.16 cfs 3,609 cf
Subcatchment9S: N SWALE 2	Runoff Area=14,101 sf 0.00% Impervious Runoff Depth=3.02"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=83 Runoff=1.13 cfs 3,550 cf
Subcatchment 10S: N SWALE 3	Runoff Area=12,739 sf 0.00% Impervious Runoff Depth=3.02"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=83 Runoff=1.03 cfs 3,207 cf
Subcatchment11S: NW PERF. DRAIN	Runoff Area=45,853 sf 0.00% Impervious Runoff Depth=1.40"
Flow Length=660'	Slope=0.0200 '/' Tc=6.0 min CN=63 Runoff=1.60 cfs 5,358 cf
Subcatchment 12S: CENTRAL PERF. DRAI	N Runoff Area=6,658 sf 0.00% Impervious Runoff Depth=1.40" Tc=5.0 min CN=63 Runoff=0.24 cfs 778 cf
Subcatchment13S: GIS BUILDINGS	Runoff Area=21,900 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=2.46 cfs 8,383 cf
Subcatchment 14S: GIS BLDGS PERF	Runoff Area=74,801 sf 0.00% Impervious Runoff Depth=1.40"
Flow Length=570'	Slope=0.0200 '/' Tc=8.4 min CN=63 Runoff=2.39 cfs 8,741 cf
	N Runoff Area=54,087 sf 0.00% Impervious Runoff Depth=1.40" Slope=0.0200 '/' Tc=8.4 min CN=63 Runoff=1.73 cfs 6,321 cf
Subcatchment 16S: N PERF. DRAIN	Runoff Area=35,583 sf 0.57% Impervious Runoff Depth=1.40" Tc=5.0 min CN=63 Runoff=1.29 cfs 4,158 cf

CWW Substation 5-Parcel Proposed ConditiType III 24-hr NRCC 10YR 24H Rainfall=4.83"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 42				
Subcatchment 17S: SE PERF. DRAIN 1 Flow Length=425'	Runoff Area=41,546 sf 20.00% Impervious Runoff Depth=1.91" Slope=0.0200 '/' Tc=5.9 min CN=70 Runoff=2.10 cfs 6,617 cf			
Subcatchment 18S: SE PERF. DRAIN 2	Runoff Area=25,304 sf 16.82% Impervious Runoff Depth=1.83" Tc=5.0 min CN=69 Runoff=1.26 cfs 3,869 cf			
Subcatchment20S: SR4	Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.20 cfs 668 cf			
Subcatchment21S: XFMR 1	Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.30 cfs 1,031 cf			
Subcatchment22S: SR5	Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.20 cfs 668 cf			
Subcatchment23S: XFMR 2	Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.30 cfs 1,031 cf			
Subcatchment24S: XFMR 3	Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.30 cfs 1,031 cf			
Subcatchment25S: SR6	Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.20 cfs 668 cf			
Subcatchment26S: SR1	Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.18 cfs 612 cf			
Subcatchment27S: SR2	Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.18 cfs 612 cf			
Subcatchment28S: SR3	Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.18 cfs 612 cf			
Subcatchment29S: STATCOM1	Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.20 cfs 678 cf			
Subcatchment30S: STATCOM2	Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.20 cfs 678 cf			
Subcatchment31S: STATCOM3	Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=4.59" Tc=5.0 min CN=98 Runoff=0.20 cfs 678 cf			
	vg. Flow Depth=0.29' Max Vel=1.56 fps Inflow=0.45 cfs 1,743 cf 9.0' S=0.0137 '/' Capacity=10.56 cfs Outflow=0.40 cfs 1,743 cf			
	vg. Flow Depth=0.27' Max Vel=1.76 fps Inflow=0.40 cfs 2,470 cf 7.3' S=0.0195 '/' Capacity=12.62 cfs Outflow=0.38 cfs 2,470 cf			
	vg. Flow Depth=0.32' Max Vel=1.69 fps Inflow=0.55 cfs 3,720 cf 9.0' S=0.0142 '/' Capacity=10.77 cfs Outflow=0.53 cfs 3,720 cf			
	vg. Flow Depth=0.58' Max Vel=1.79 fps Inflow=1.91 cfs 9,800 cf 67.8' S=0.0073 '/' Capacity=9.98 cfs Outflow=1.80 cfs 9,800 cf			

CWW Substation 5-Parcel Proposed Conditi Type III 24-hr NRCC	10YR 24H Rainfall=4.83"
Prepared by Stantec Consulting Ltd.	Printed 2/10/2023
HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLC	Page 43

	NEL Avg. Flow Depth=0.21' Max Vel=3.65 fps Inflow=15.16 cfs 57,843 cf 0.078 L=175.0' S=0.3086 '/' Capacity=198.63 cfs Outflow=15.05 cfs 57,843 cf
Reach 11R: N SWALE PART	Avg. Flow Depth=0.41' Max Vel=2.06 fps Inflow=1.16 cfs 3,609 cf n=0.030 L=447.0' S=0.0153 '/' Capacity=11.19 cfs Outflow=1.04 cfs 3,609 cf
Reach 12R: N SWALE PART	2 Avg. Flow Depth=0.53' Max Vel=2.32 fps Inflow=2.12 cfs 7,159 cf n=0.030 L=431.0' S=0.0137 '/' Capacity=10.59 cfs Outflow=1.97 cfs 7,159 cf
Reach 13R: N SWALE PART	3 Avg. Flow Depth=0.61' Max Vel=2.47 fps Inflow=2.88 cfs 10,366 cf n=0.030 L=386.0' S=0.0131 '/' Capacity=10.33 cfs Outflow=2.75 cfs 10,366 cf
	- LOCALIZED Peak Elev=87.70' Storage=32,038 cf Inflow=15.05 cfs 57,994 cf Discarded=0.90 cfs 57,994 cf Primary=0.00 cfs 0 cf Outflow=0.90 cfs 57,994 cf
Pond 2P: SEDIMENT FOREB Disca	AY Peak Elev=150.06' Storage=8,963 cf Inflow=4.55 cfs 20,165 cf arded=0.14 cfs 15,515 cf Primary=0.68 cfs 4,651 cf Outflow=0.82 cfs 20,166 cf
Pond 3P: DMHs w/ Vortex Un	it Peak Elev=149.52' Inflow=15.16 cfs 53,192 cf 36.0" Round Culvert n=0.011 L=25.0' S=0.1200 '/' Outflow=15.16 cfs 53,192 cf
Pond 4P: DMH	Peak Elev=151.82' Inflow=15.16 cfs 53,192 cf 36.0" Round Culvert n=0.011 L=36.0' S=0.0639 '/' Outflow=15.16 cfs 53,192 cf
Pond 5P: DMH	Peak Elev=156.80' Inflow=9.41 cfs 34,679 cf 24.0" Round Culvert n=0.011 L=263.0' S=0.0186 '/' Outflow=9.41 cfs 34,679 cf
Pond 6P: OIL/WATER SEPAR	Peak Elev=156.85' Inflow=1.50 cfs 5,098 cf 24.0" Round Culvert n=0.012 L=27.0' S=0.0148 '/' Outflow=1.50 cfs 5,098 cf
Pond 7P: DMH	Peak Elev=157.17' Inflow=1.50 cfs 5,098 cf 12.0" Round Culvert n=0.011 L=40.0' S=0.0100 '/' Outflow=1.50 cfs 5,098 cf
Pond 8P: DMH	Peak Elev=158.05' Inflow=1.00 cfs 3,398 cf 12.0" Round Culvert n=0.011 L=154.0' S=0.0065 '/' Outflow=1.00 cfs 3,398 cf
Pond 9P: DMH	Peak Elev=159.65' Inflow=0.50 cfs 1,699 cf 12.0" Round Culvert n=0.011 L=141.0' S=0.0092 '/' Outflow=0.50 cfs 1,699 cf
Pond 10P: 24" Petro-Barrier	Peak Elev=162.70' Inflow=0.20 cfs 668 cf 6.0" Round Culvert n=0.010 L=30.0' S=0.0143 '/' Outflow=0.20 cfs 668 cf
Pond 11P: 24" Petro-Barrier	Peak Elev=159.98' Inflow=0.30 cfs 1,031 cf 6.0" Round Culvert n=0.010 L=18.0' S=0.0122 '/' Outflow=0.30 cfs 1,031 cf
Pond 12P: 24" Petro-Barrier	Peak Elev=160.97' Inflow=0.20 cfs 668 cf 6.0" Round Culvert n=0.010 L=32.0' S=0.0094 '/' Outflow=0.20 cfs 668 cf
Pond 13P: 24" Petro-Barrier	Peak Elev=158.25' Inflow=0.30 cfs 1,031 cf 6.0" Round Culvert n=0.010 L=20.0' S=0.0115 '/' Outflow=0.30 cfs 1,031 cf

CWW Substation 5-Parcel Proposed Conditi Type III 24-hr NRCC 10YR 24H Rainfall=4.83" Prepared by Stantec Consulting Ltd. Printed 2/10/2023 HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLC Page 44 Peak Elev=157.32' Inflow=0.30 cfs 1,031 cf Pond 14P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=21.0' S=0.0133 '/' Outflow=0.30 cfs 1,031 cf Peak Elev=157.94' Inflow=0.20 cfs 668 cf Pond 15P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=34.0' S=0.0076 '/' Outflow=0.20 cfs 668 cf Peak Elev=159.74' Inflow=1.84 cfs 6,136 cf Pond 16P: DMH 12.0" Round Culvert n=0.011 L=17.0' S=0.0294 '/' Outflow=1.84 cfs 6,136 cf Peak Elev=160.49' Inflow=6.20 cfs 23.445 cf Pond 17P: DMH 24.0" Round Culvert n=0.011 L=88.2' S=0.0442 '/' Outflow=6.20 cfs 23,445 cf Peak Elev=162.48' Inflow=4.12 cfs 15,062 cf Pond 18P: DMH 18.0" Round Culvert n=0.011 L=92.0' S=0.0217 '/' Outflow=4.12 cfs 15,062 cf Pond 20P: DMH Peak Elev=152.92' Inflow=4.64 cfs 14,644 cf 24.0" Round Culvert n=0.011 L=56.0' S=0.0286 '/' Outflow=4.64 cfs 14,644 cf Peak Elev=155.53' Inflow=1.26 cfs 3,869 cf Pond 21P: DMH 12.0" Round Culvert n=0.011 L=62.0' S=0.0410 '/' Outflow=1.26 cfs 3,869 cf Pond 22P: DMH Peak Elev=156.39' Inflow=3.38 cfs 10,775 cf 18.0" Round Culvert n=0.011 L=56.0' S=0.0089 '/' Outflow=3.38 cfs 10,775 cf Peak Elev=152.00' Inflow=1.14 cfs 3,870 cf Pond 23P: OIL/WATER SEPARATOR 12.0" Round Culvert n=0.011 L=182.0' S=0.0093 '/' Outflow=1.18 cfs 3,869 cf Pond 24P: DMH Peak Elev=152.07' Inflow=1.14 cfs 3,870 cf 12.0" Round Culvert n=0.011 L=1.0' S=0.1000 '/' Outflow=1.14 cfs 3,870 cf Peak Elev=152.13' Inflow=0.20 cfs 678 cf Pond 25P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=58.0' S=0.0102 '/' Outflow=0.20 cfs 678 cf Pond 26P: 24" Petro-Barrier Peak Elev=153.26' Inflow=0.20 cfs 678 cf 6.0" Round Culvert n=0.010 L=17.0' S=0.0576 '/' Outflow=0.20 cfs 678 cf Peak Elev=154.34' Inflow=0.74 cfs 2,515 cf Pond 27P: DMH 12.0" Round Culvert n=0.011 L=224.0' S=0.0085 '/' Outflow=0.74 cfs 2,515 cf Pond 28P: DMH Peak Elev=154.84' Inflow=0.74 cfs 2,515 cf 12.0" Round Culvert n=0.011 L=50.0' S=0.0080 '/' Outflow=0.74 cfs 2,515 cf Peak Elev=155.23' Inflow=0.20 cfs 678 cf Pond 29P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=7.0' S=0.0643 '/' Outflow=0.20 cfs 678 cf Peak Elev=160.77' Inflow=0.54 cfs 1,837 cf Pond 30P: DMH 12.0" Round Culvert n=0.011 L=98.0' S=0.0092 '/' Outflow=0.54 cfs 1,837 cf Peak Elev=162.27' Inflow=0.54 cfs 1,837 cf Pond 31P: DMH 12.0" Round Culvert n=0.011 L=232.0' S=0.0060 '/' Outflow=0.54 cfs 1,837 cf Peak Elev=162.70' Inflow=0.18 cfs 612 cf Pond 32P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=10.0' S=0.0240 '/' Outflow=0.18 cfs 612 cf

Pond 33P: DMH	Peak Elev=163.50' Inflow=0.36 cfs 1,225 cf 12.0" Round Culvert n=0.011 L=159.0' S=0.0075 '/' Outflow=0.36 cfs 1,225 cf
Pond 34P: 24" Petro-Barri	er Peak Elev=163.69' Inflow=0.18 cfs 612 cf 6.0" Round Culvert n=0.010 L=8.0' S=0.0162 '/' Outflow=0.18 cfs 612 cf
Pond 35P: 24" Petro-Barri	er Peak Elev=164.65' Inflow=0.18 cfs 612 cf 6.0" Round Culvert n=0.010 L=38.0' S=0.0103 '/' Outflow=0.18 cfs 612 cf
Pond 40P: DMH	Peak Elev=158.40' Inflow=1.35 cfs 7,429 cf 24.0" x 12.0" Box Culvert n=0.011 L=10.0' S=0.0300 '/' Outflow=1.35 cfs 7,429 cf
Pond 41P: CULVERT	Peak Elev=159.19' Inflow=0.53 cfs 3,720 cf 24.0" x 12.0" Box Culvert n=0.011 L=55.0' S=0.0182 '/' Outflow=0.53 cfs 3,720 cf
Link 1L: OVERFLOW	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Link 10L: OFF-SITE	Inflow=0.01 cfs 332 cf Primary=0.01 cfs 332 cf

Total Runoff Area = 1,041,520 sf Runoff Volume = 73,841 cf Average Runoff Depth = 0.85" 94.42% Pervious = 983,426 sf 5.58% Impervious = 58,094 sf

Summary for Subcatchment 1S: AREA OUTSIDE SUBSTATION - NOT TO POND

Runoff = 0.01 cfs @ 22.05 hrs, Volume= 332 cf, Depth= 0.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

Area (sf) CN Description	Description			
179,269 30 Woods, Good, HSG A				
72,994 30 Meadow, non-grazed, HSG A				
6,387 96 Gravel surface, HSG A				
258,650 32 Weighted Average				
258,650 100.00% Pervious Area				
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)				
5.0 Direct Entry, Direct Entry				

Summary for Subcatchment 2S: AREA OUTSIDE SUBSTATION - TO POND

Runoff = 0.01 cfs @ 23.63 hrs, Volume= 151 cf, Depth= 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

A	rea (sf)	CN [Description				
2	206,991	30 V	Voods, Go	od, HSG A			
	72,294	30 N	Meadow, no	on-grazed,	HSG A		
	6,347	96 (Gravel surfa	ace, HSG A	Ι		
2	85,632	31 V	Veighted A	verage			
2	85,632	1	100.00% Pe	ervious Are	a		
Тс	Length	Slope	Velocity	Capacity	Description		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
3.4	50	0.5000	0.25		Sheet Flow, Sheet		
					Woods: Light underbrush n= 0.400 P2= 3.29"		
8.9	441	0.1100	0.83		Shallow Concentrated Flow, Shallow Conc		
					Forest w/Heavy Litter Kv= 2.5 fps		
12.3	491	Total					

Summary for Subcatchment 3S: S SWALE 1

Runoff = 0.45 cfs @ 12.11 hrs, Volume= 1,743 cf, Depth= 0.96"

	A	rea (sf)	CN	Description				
*		3,599	63	Crushed Stone Surface, HSG A				
		6,746	96	Gravel surfa	ace, HSG A	A		
		7,202	30	Woods, Go	od, HSG A			
		4,354	30	Meadow, no	on-grazed,	HSG A		
		21,901	56	Weighted A	verage			
		21,901		100.00% Pe	ervious Are	a		
	Тс	Length	Slope		Capacity	Description		
	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)			
	2.3	50	0.0200	0.36		Sheet Flow, Sheet		
						Fallow n= 0.050 P2= 3.29"		
	3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc		
_						Nearly Bare & Untilled Kv= 10.0 fps		
	6.2	378	Total					
				Summar	y for Sub	catchment 4S: S SWALE 2		
					-			

Runoff = 0.05 cfs @ 12.42 hrs, Volume=

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

	Area (sf)	CN	Description				
	5,160	96	Gravel surface, HSG A				
*	2,578	63	Crushed St	one Surfac	e, HSG A		
	20,025	30	Woods, Go	od, HSG A			
	9,644	30	Meadow, no	on-grazed,	HSG A		
	37,407	41	41 Weighted Average				
	37,407		100.00% Pe	ervious Are	а		
-				0			
TC	5	Slope	,	Capacity	Description		
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)			
5.0					Direct Entry, Direct Entry		

727 cf, Depth= 0.23"

Summary for Subcatchment 5S: S SWALE 3

Runoff = 0.17 cfs @ 12.27 hrs, Volume= 1,250 cf, Depth= 0.53"

	Area (sf)	CN	Description
*	3,335	63	Crushed Stone Surface, HSG A
	7,061	30	Woods, Good, HSG A
	6,315	96	Gravel surface, HSG A
	11,820	30	Meadow, non-grazed, HSG A
	28,531	48	Weighted Average
	28,531		100.00% Pervious Area

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
						Fallow n= 0.050 P2= 3.29"
	7.0	594	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps
-						Nearly Date & Onlined INV- 10.0 1p3
	9.3	644	Total			

Summary for Subcatchment 6S: ACCESS RAMP

Runoff = 1.24 cfs @ 12.08 hrs, Volume= 3,709 cf, Depth= 2.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

Area (sf) CN	Description		
12,617	7 96	Gravel surfa	ace, HSG A	4
1,094	4 30	Woods, Go	od, HSG A	
3,620) 30	Meadow, no	on-grazed,	HSG A
17,331	1 78	Weighted A	verage	
17,331	1	100.00% Pe	ervious Are	a
Tc Lengt	th Slop	pe Velocity	Capacity	Description
(min) (fee	et) (ft/	ft) (ft/sec)	(cfs)	
5.0				Direct Entry, Direct Entry
				•••••••
		-		

Summary for Subcatchment 7S: S SWALE 4

Runoff = 0.63 cfs @ 12.14 hrs, Volume= 2,371 cf, Depth= 1.40"

_	A	rea (sf)	CN [Description					
*		6,230	63 (63 Crushed Stone Surface, HSG A					
		7,051	96 (Gravel surfa	ace, HSG A	N Contraction of the second seco			
_		7,010	30 N	/leadow, no	on-grazed,	HSG A			
	20,291 63 Weighted Average								
		20,291	1	00.00% Pe	ervious Are	а			
	Тс	Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	2.3	50	0.0200	0.36		Sheet Flow, Sheet			
						Fallow n= 0.050 P2= 3.29"			
	7.0	594	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc			
						Nearly Bare & Untilled Kv= 10.0 fps			
	9.3	644	Total						

Summary for Subcatchment 8S: N SWALE 1

Runoff = 1.16 cfs @ 12.09 hrs, Volume= 3,609 cf, Depth= 2.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

	A	rea (sf)	CN [Description					
*		4,949	63 (63 Crushed Stone Surface, HSG A					
		9,543	96 (Gravel surfa	ace, HSG A	A Í			
		1,286	30 N	Meadow, no	on-grazed,	HSG A			
		15,778	80 \	Neighted A	verage				
		15,778		100.00% P	ervious Are	a			
		Length	Slope	,	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	2.3	50	0.0200	0.36		Sheet Flow, Sheet			
						Fallow n= 0.050 P2= 3.29"			
	3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc			
						Nearly Bare & Untilled Kv= 10.0 fps			
	6.0	270	Tatal						

6.2 378 Total

Summary for Subcatchment 9S: N SWALE 2

Runoff = 1.13 cfs @ 12.09 hrs, Volume= 3,550 cf, Depth= 3.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

_	A	rea (sf)	CN I	Description				
*		5,475	63 (63 Crushed Stone Surface, HSG A				
_		8,626	96 (Gravel surfa	ace, HSG A	Α		
	14,101 83 Weighted Average							
	14,101			100.00% Pervious Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	2.3	50	0.0200	0.36		Sheet Flow, Sheet		
	3.9	328	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps		
_		0 - 0						

6.2 378 Total

Summary for Subcatchment 10S: N SWALE 3

Runoff = 1.03 cfs @ 12.09 hrs, Volume= 3,207 cf, Depth= 3.02"

_	A	rea (sf)	CN [Description				
*		4,958	63 (63 Crushed Stone Surface, HSG A				
_		7,781	96 (Gravel surfa	ace, HSG A	λ		
		12,739	83 \	Veighted A	verage			
		12,739		00.00% Pe	ervious Are	а		
	Tc	Length	Slope		Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	2.3	50	0.0200	0.36		Sheet Flow, Sheet		
						Fallow n= 0.050 P2= 3.29"		
	3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc		
						Nearly Bare & Untilled Kv= 10.0 fps		
_	6.2	378	Total					

Summary for Subcatchment 11S: NW PERF. DRAIN

Runoff = 1.60 cfs @ 12.10 hrs, Volume= 5,358 cf, Depth= 1.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

	A	rea (sf)	CN E	Description		
*		45,853	63 C	Crushed St	one Surface	e, HSG A
		45,853	1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
	3.0	255	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps
	0.7	355	0.0200	8.34	6.55	Pipe Channel, Perf Pipe
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
	6.0	660	Total			·

Summary for Subcatchment 12S: CENTRAL PERF. DRAIN 2

Runoff = 0.24 cfs @ 12.08 hrs, Volume= 778 cf, Depth= 1.40"

	Area (sf)	CN	Description
*	6,658	63	Crushed Stone Surface, HSG A
	6,658		100.00% Pervious Area

Prepare	CWW Substation 5-Parcel Proposed ConditiType III 24-hr NRCC 10YR 24H Rainfall=4.83"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 51									
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description					
5.0					Direct Entry, Direct Entry					
	Summary for Subcatchment 13S: GIS BUILDINGS									
Runoff	=	2.46 c	fs @ 12.0 [*]	7 hrs, Volu	Ime= 8,383 cf, Depth= 4.59"					
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"									
A	rea (sf)		Description							
	21,900		Roofs, HSG							
	21,900		100.00% Im	pervious A	Area					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description					
5.0					Direct Entry, Direct Entry					
	;	Summa	ary for Su	ıbcatchm	nent 14S: GIS BLDGS PERF DRAIN					
Runoff	=	2.39 c	fs @ 12.1	3 hrs, Volu	ume= 8,741 cf, Depth= 1.40"					
			hod, UH=S R 24H Rair		ted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs					
A	rea (sf)	CN I	Description							
*	74,801		Crushed St	one Surface	e, HSG A					
	74,801		100.00% Pe	ervious Are	a					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
2.3	50	0.0200			Sheet Flow, Sheet					
6.1	520	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps					
8.4	570	Total								
	5	Summa	ry for Su	bcatchm	ent 15S: CENTRAL PERF. DRAIN 1					
Runoff	=	1.73 c	fs @ 12.1	3 hrs, Volu	ume= 6,321 cf, Depth= 1.40"					

	Area (sf)	CN	Description
*	54,087	63	Crushed Stone Surface, HSG A
	54,087		100.00% Pervious Area

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
	6.1	520	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc
	0.1	520	0.0200	1.41		Nearly Bare & Untilled Kv= 10.0 fps
_	8.4	570	Total			

Summary for Subcatchment 16S: N PERF. DRAIN

Runoff = 1.29 cfs @ 12.08 hrs, Volume= 4,158 cf, Depth= 1.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

	A	rea (sf)	CN	Description		
*		35,381 202	63 98	Crushed St Roofs, HSC		e, HSG A
		35,583 35,381 202	63	Weighted A 99.43% Per 0.57% Impe	vious Area	
	Tc (min)	Length (feet)	Slop (ft/ff	,	Capacity (cfs)	Description
_	5.0					Direct Entry, Direct Entry

Summary for Subcatchment 17S: SE PERF. DRAIN 1

Runoff = 2.10 cfs @ 12.09 hrs, Volume= 6,617 cf, Depth= 1.91"

_	A	rea (sf)	CN	Description		
		8,310	98	Roofs, HSC	βA	
*		33,236	63	Crushed St	one Surfac	e, HSG A
		41,546	70	Weighted A	verage	
		33,236			rvious Area	
		8,310	:	20.00% Imp	pervious Are	ea
	Тс	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
						Fallow n= 0.050 P2= 3.29"
	3.5	300	0.0200	1.41		Shallow Concentrated Flow, Shallow
						Nearly Bare & Untilled Kv= 10.0 fps
	0.1	75	0.0200	8.34	6.55	Pipe Channel, Perf. Pipe
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
_						n= 0.010 PVC, smooth interior
	5.9	425	Total			

Summary for Subcatchment 18S: SE PERF. DRAIN 2

Runoff = 1.26 cfs @ 12.08 hrs, Volume= 3,869 cf, Depth= 1.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

	Area ((sf)	CN	Description		
	4,2	255	98	Roofs, HSC	βA	
*	21,0)49	63	Crushed St	one Surfac	e, HSG A
	25,3 21,0 4,2)49 255		Weighted A 83.18% Pei 16.82% Imp	rvious Area pervious Are	ea
(5		ngth eet)	Slope		Capacity	Description
(I		eel)	(ft/ft) (ft/sec)	(cfs)	
	5.0					Direct Entry, Direct Entry

Summary for Subcatchment 20S: SR4

Runoff = 0.20 cfs @ 12.07 hrs, Volume= 668 cf, Depth= 4.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

A	rea (sf)	CN I	Description					
	1,745	98 (98 Concrete Containment					
	1,745	745 100.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry, Direct Entry			

Summary for Subcatchment 21S: XFMR 1

Runoff = 0.30 cfs @ 12.07 hrs, Volume= 1,031 cf, Depth= 4.59"

A	rea (sf)	CN E	Description						
	2,694	98 C	08 Concrete Containment						
	2,694	1	100.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry, Direct Entry				

Summary for Subcatchment 22S: SR5

Runoff = 0.20 cfs @ 12.07 hrs, Volume= 668 cf, Depth= 4.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

Area (sf) CN Description										
1,745 98 Concrete Containment										
1,745 100.00% Impervious Area										
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
5.0 Direct Entry, Direct Entry										
Summary for Subcatchment 23S: XFMR 2										
Runoff = 0.30 cfs @ 12.07 hrs, Volume= 1,031 cf, Depth= 4.59"										
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"										
Area (sf) CN Description										
2,694 98 Concrete Containment										
2,694 100.00% Impervious Area										
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)										
5.0 Direct Entry, Direct Entry										
Summary for Subcatchment 24S: XFMR 3										
Runoff = 0.30 cfs @ 12.07 hrs, Volume= 1,031 cf, Depth= 4.59"										
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"										
Area (sf) CN Description										
2,694 98 Concrete Containment										
2,694 100.00% Impervious Area										

Description

Direct Entry, Direct Entry

(cfs)

Slope Velocity Capacity

(ft/sec)

Tc

(min)

5.0

Length

(feet)

(ft/ft)

Summary for Subcatchment 25S: SR6

Runoff = 0.20 cfs @ 12.07 hrs, Volume= 668 cf, Depth= 4.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"

A	rea (sf)	CN	Description							
	1,745	98	Concrete Co	ontainment						
	1,745		100.00% Im	pervious A	rea					
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description					
5.0	()	(1211)	((0.0)	Direct Entry	, Direct Entry				
	Summary for Subcatchment 26S: SR1									
Runoff	=	0.18 c	fs @ 12.0 ⁻	7 hrs, Volu	ime=	612 cf, Depth= 4.59"				
			thod, UH=S R 24H Rain		ted-CN, Time S	Span= 0.00-72.00 hrs, dt= 0.01 hrs				
A	rea (sf)	CN	Description							
	1,600	98	Concrete Co	ontainment						
	1,600		100.00% Im	pervious A	rea					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description					
5.0					Direct Entry,	, Direct Entry				
			Sumn	nary for S	Subcatchme	ent 27S: SR2				
Runoff	=	0.18 c	fs @ 12.0	7 hrs, Volu	ime=	612 cf, Depth= 4.59"				
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"									
A	rea (sf)		Description							
	1,600		Concrete Co							
	1,600		100.00% Im	pervious A	rea					
Тс	Length	Slope	Velocity	Capacity	Description					

(feet)

(min)

5.0

(ft/ft)

(ft/sec)

(cfs)

Direct Entry, Direct Entry

Summary for Subcatchment 28S: SR3

Runoff = 0.18 cfs @ 12.07 hrs, Volume= 612 cf, Depth= 4.59"

A	rea (sf)	CN I	Description							
	1,600	98	Concrete C	ontainment	t					
	1,600		100.00% Im	npervious A	rea					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description					
5.0					Direct Entry,	Direct Entry				
	Summary for Subcatchment 29S: STATCOM 1 TRANSFORMER									
Runoff	=	0.20 c	fs @ 12.0 ⁻	7 hrs, Volu	ume=	678 cf, Depth= 4.59"				
			thod, UH=S R 24H Rair		ited-CN, Time S	Span= 0.00-72.00 hrs, dt= 0.01 hrs				
A	rea (sf)	CN I	Description							
	1,770	98	Concrete C	ontainment	t					
	1,770		100.00% Im	npervious A	rea					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description					
5.0					Direct Entry,	Direct Entry				
	Summary for Subcatchment 30S: STATCOM 2 TRANSFORMER									
Runoff	=	0.20 c	fs @ 12.0 ⁻	7 hrs, Volu	ıme=	678 cf, Depth= 4.59"				
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 10YR 24H Rainfall=4.83"									
A	rea (sf)		Description							
	1,770	98	Concrete C	ontainment	t					
	1,770		100.00% Im	npervious A	rea					

Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	-
5.0					Direct Entry, Direct Entry

Summary for Subcatchment 31S: STATCOM 3 TRANSFORMER

Runoff = 0.20 cfs @ 12.07 hrs, Volume= 678 cf, Depth= 4.59"

Area (sf) CN Description									
1,770 98 Concrete Containment									
1,770 100.00% Impervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
5.0 Direct Entry, Direct Entry									
Summary for Reach 1R: S SWALE PART 1									
Inflow Area = 21,901 sf, 0.00% Impervious, Inflow Depth = 0.96" for NRCC 10YR 24H event Inflow = 0.45 cfs @ 12.11 hrs, Volume= 1,743 cf Outflow = 0.40 cfs @ 12.15 hrs, Volume= 1,743 cf, Atten= 10%, Lag= 2.6 min									
Outhow $-$ 0.40 cis @ 12.15 ms, volume- 1,745 ci, Atten- 10%, Lag- 2.0 mm									
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 1.56 fps, Min. Travel Time= 3.3 min Avg. Velocity = 0.73 fps, Avg. Travel Time= 7.1 min									
Peak Storage= 80 cf @ 12.15 hrs Average Depth at Peak Storage= 0.29' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 38.49 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.56 cfs									
0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 309.0' Slope= 0.0137 '/' Inlet Invert= 174.13', Outlet Invert= 169.91'									

Summary for Reach 2R: S SWALE PART 2

Inflow Area	a =	59,308 sf, 0.00% Impervious, Inflow Depth = 0.50" for NRCC 10YR 24	4H event
Inflow	=	0.40 cfs @ 12.15 hrs, Volume= 2,470 cf	
Outflow	=	0.38 cfs @ 12.19 hrs, Volume= 2,470 cf, Atten= 5%, Lag= 2.3 mir	า

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 1.76 fps, Min. Travel Time= 2.4 min Avg. Velocity = 0.92 fps, Avg. Travel Time= 4.7 min

Peak Storage= 56 cf @ 12.19 hrs Average Depth at Peak Storage= 0.27' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 46.01 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 12.62 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 257.3' Slope= 0.0195 '/' Inlet Invert= 169.90', Outlet Invert= 164.88'

Summary for Reach 3R: S SWALE PART 3

Inflow Are	a =	87,839 sf, 0.00% Impervious	s, Inflow Depth = 0.51" for NRCC 10YR 24H event
Inflow	=	0.55 cfs @ 12.20 hrs, Volume	= 3,720 cf
Outflow	=	0.53 cfs @ 12.27 hrs, Volume	= 3,720 cf, Atten= 5%, Lag= 4.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 1.69 fps, Min. Travel Time= 3.2 min Avg. Velocity = 0.87 fps, Avg. Travel Time= 6.3 min

Peak Storage= 103 cf @ 12.27 hrs Average Depth at Peak Storage= 0.32' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 39.24 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.77 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 329.0' Slope= 0.0142 '/' Inlet Invert= 164.80', Outlet Invert= 160.13'

Summary for Reach 4R: S SWALE PART 4

Inflow Area = 125,461 sf, 0.00% Impervious, Inflow Depth = 0.94" for NRCC 10YR 24H event 1.91 cfs @ 12.11 hrs. Volume= Inflow 9.800 cf = 1.80 cfs @ 12.16 hrs, Volume= Outflow = 9,800 cf, Atten= 6%, Lag= 3.2 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 1.79 fps, Min. Travel Time= 3.4 min Avg. Velocity = 0.74 fps, Avg. Travel Time= 8.3 min Peak Storage= 369 cf @ 12.16 hrs Average Depth at Peak Storage= 0.58' Defined Flood Depth= 2.00' Flow Area= 9.5 sf, Capacity= 31.59 cfs Bank-Full Depth= 1.10' Flow Area= 3.6 sf, Capacity= 9.98 cfs 0.00' x 1.10' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.60'

Length= 367.8' Slope= 0.0073 '/' Inlet Invert= 157.70', Outlet Invert= 155.00'

Summary for Reach 5R: DRAINAGE CHANNEL

 Inflow Area =
 497,238 sf, 11.68% Impervious, Inflow Depth =
 1.40" for NRCC 10YR 24H event

 Inflow =
 15.16 cfs @
 12.09 hrs, Volume=
 57,843 cf

 Outflow =
 15.05 cfs @
 12.10 hrs, Volume=
 57,843 cf, Atten= 1%, Lag= 0.5 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 3.65 fps, Min. Travel Time= 0.8 min Avg. Velocity = 0.87 fps, Avg. Travel Time= 3.4 min

Peak Storage= 722 cf @ 12.10 hrs Average Depth at Peak Storage= 0.21' Defined Flood Depth= 1.00' Flow Area= 20.0 sf, Capacity= 198.63 cfs Bank-Full Depth= 1.00' Flow Area= 20.0 sf, Capacity= 198.63 cfs

20.00' x 1.00' deep channel, n= 0.078 Riprap, 12-inch Length= 175.0' Slope= 0.3086 '/' Inlet Invert= 145.00', Outlet Invert= 91.00'

Summary for Reach 11R: N SWALE PART 1

Inflow Area = 15.778 sf. 0.00% Impervious. Inflow Depth = 2.75" for NRCC 10YR 24H event Inflow 1.16 cfs @ 12.09 hrs. Volume= 3.609 cf = 1.04 cfs @ 12.13 hrs, Volume= Outflow = 3,609 cf, Atten= 10%, Lag= 2.4 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.06 fps, Min. Travel Time= 3.6 min Avg. Velocity = 0.80 fps, Avg. Travel Time= 9.3 min Peak Storage= 226 cf @ 12.13 hrs Average Depth at Peak Storage= 0.41' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 40.78 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 11.19 cfs 0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 447.0' Slope= 0.0153 '/' Inlet Invert= 174.03', Outlet Invert= 167.18'

Summary for Reach 12R: N SWALE PART 2

 Inflow Area =
 29,879 sf,
 0.00% Impervious,
 Inflow Depth =
 2.88"
 for
 NRCC 10YR 24H event

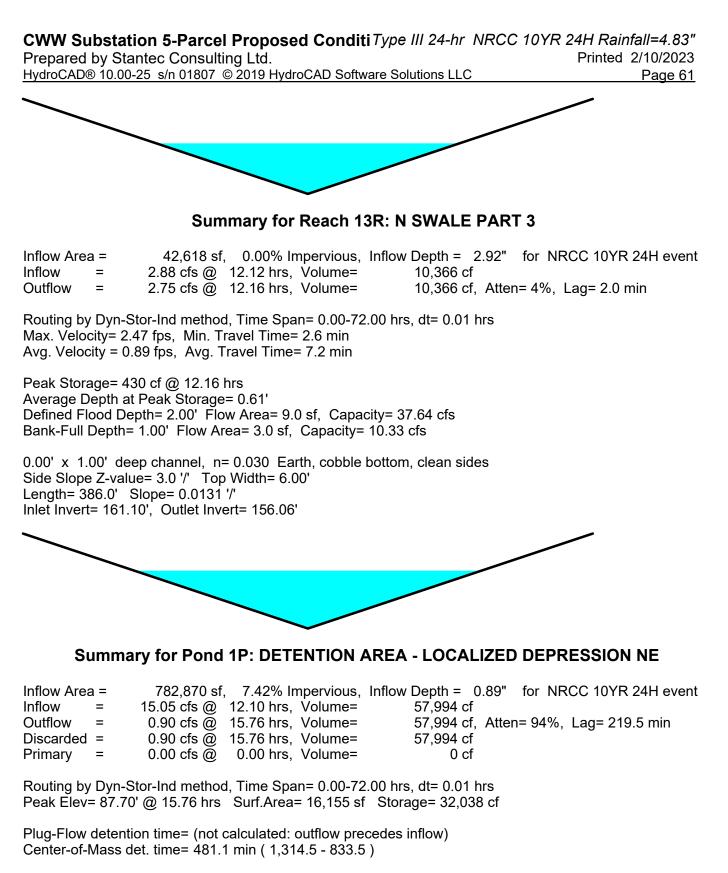
 Inflow =
 2.12 cfs @
 12.11 hrs,
 Volume=
 7,159 cf

 Outflow =
 1.97 cfs @
 12.14 hrs,
 Volume=
 7,159 cf,
 Atten= 7%,
 Lag= 2.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.32 fps, Min. Travel Time= 3.1 min Avg. Velocity = 0.85 fps, Avg. Travel Time= 8.4 min

Peak Storage= 367 cf @ 12.14 hrs Average Depth at Peak Storage= 0.53' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 38.60 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.59 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 431.0' Slope= 0.0137 '/' Inlet Invert= 167.10', Outlet Invert= 161.18'



Volume	Invert	Avail.Storage	Storage Description
#1	83.00'	125,742 cf	Custom Stage Data (Irregular)Listed below (Recalc)

CWW Substation 5-Parcel Proposed Conditi*Type III 24-hr NRCC 10YR 24H Rainfall=4.83"* Prepared by Stantec Consulting Ltd. Printed 2/10/2023

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Page 62

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
83.00	827	135.0	0	0	827
84.00	2,756	217.0	1,698	1,698	3,131
85.00	5,016	298.0	3,830	5,528	6,460
86.00	8,057	383.0	6,477	12,004	11,079
87.00	12,347	488.0	10,126	22,130	18,370
88.00	17,968	589.0	15,070	37,200	27,043
89.00	25,096	690.0	21,433	58,633	37,342
90.00	33,858	865.0	29,368	88,001	59,011
91.00	41,761	936.1	37,740	125,742	69,242

Device	Routing	Invert	Outlet Devices
#1	Discarded	83.00'	2.410 in/hr Exfiltration over Surface area
#2	Primary	90.60'	50.0' long x 4.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66
			2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32

Discarded OutFlow Max=0.90 cfs @ 15.76 hrs HW=87.70' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.90 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=83.00' TW=0.00' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: SEDIMENT FOREBAY

Inflow Area =	168,079 sf, 0.00% Impervious,	Inflow Depth = 1.44" for NRCC 10YR 24H event
Inflow =	4.55 cfs @ 12.16 hrs, Volume=	20,165 cf
Outflow =	0.82 cfs @ 12.96 hrs, Volume=	20,166 cf, Atten= 82%, Lag= 48.1 min
Discarded =	0.14 cfs @ 12.96 hrs, Volume=	15,515 cf
Primary =	0.68 cfs @ 12.96 hrs, Volume=	4,651 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 150.06' @ 12.96 hrs Surf.Area= 2,426 sf Storage= 8,963 cf Flood Elev= 151.00' Surf.Area= 2,531 sf Storage= 11,301 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 554.3 min (1,410.1 - 855.8)

Volume	Invert	Avail.Sto	rage Storage	Description		
#1	146.00'	22,34	48 cf Custom	i Stage Data (Cor	nic) Listed below (Recalc)
Elevatic (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
146.0 155.0	-	2,000 3,000	0 22,348	0 22,348	2,000 3,876	
Device	Routing	Invert	Outlet Device	S		
#1 #2	Discarded Primary	146.00' 150.00'		xfiltration over S 2.0' breadth Broa		angular Weir

> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.14 cfs @ 12.96 hrs HW=150.06' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.14 cfs)

Primary OutFlow Max=0.68 cfs @ 12.96 hrs HW=150.06' TW=145.07' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 0.68 cfs @ 0.60 fps)

Summary for Pond 3P: DMHs w/ Vortex Unit

 Inflow Area =
 329,159 sf, 17.65% Impervious, Inflow Depth =
 1.94" for NRCC 10YR 24H event

 Inflow =
 15.16 cfs @
 12.09 hrs, Volume=
 53,192 cf

 Outflow =
 15.16 cfs @
 12.09 hrs, Volume=
 53,192 cf

 Primary =
 15.16 cfs @
 12.09 hrs, Volume=
 53,192 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 149.52' @ 12.09 hrs Flood Elev= 151.05'

Device	Routing	Invert	Outlet Devices
#1	Primary	148.00'	36.0" Round Culvert L= 25.0' Ke= 0.500 Inlet / Outlet Invert= 148.00' / 145.00' S= 0.1200 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf

Primary OutFlow Max=15.15 cfs @ 12.09 hrs HW=149.52' TW=145.21' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 15.15 cfs @ 4.20 fps)

Summary for Pond 4P: DMH

 Inflow Area =
 329,159 sf, 17.65% Impervious, Inflow Depth =
 1.94" for NRCC 10YR 24H event

 Inflow =
 15.16 cfs @
 12.09 hrs, Volume=
 53,192 cf

 Outflow =
 15.16 cfs @
 12.09 hrs, Volume=
 53,192 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 15.16 cfs @
 12.09 hrs, Volume=
 53,192 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 151.82' @ 12.09 hrs Flood Elev= 158.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	150.30'	36.0" Round Culvert L= 36.0' Ke= 0.500 Inlet / Outlet Invert= 150.30' / 148.00' S= 0.0639 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf

Primary OutFlow Max=15.15 cfs @ 12.09 hrs HW=151.82' TW=149.52' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 15.15 cfs @ 4.20 fps)

Summary for Pond 5P: DMH

Inflow Area = 216,616 sf, 16.26% Impervious, Inflow Depth = 1.92" for NRCC 10YR 24H event Inflow = 9.41 cfs @ 12.10 hrs, Volume= 34,679 cf Outflow = 9.41 cfs @ 12.10 hrs, Volume= 34,679 cf, Atten= 0%, Lag= 0.0 min Primary = 9.41 cfs @ 12.10 hrs, Volume= 34,679 cf Routing by Dyn-Stor-Ind method. Time Span= 0.00-72.00 hrs. dt= 0.01 hrs.

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 156.80' @ 12.10 hrs Flood Elev= 161.58'

Device	Routing	Invert	Outlet Devices
#1	Primary	155.40'	24.0" Round Culvert L= 263.0' Ke= 0.500 Inlet / Outlet Invert= 155.40' / 150.50' S= 0.0186 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=9.40 cfs @ 12.10 hrs HW=156.79' TW=151.82' (Dynamic Tailwater) -1=Culvert (Inlet Controls 9.40 cfs @ 4.02 fps)

Summary for Pond 6P: OIL/WATER SEPARATOR

Inflow Area	=	13,317 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow =	=	1.50 cfs @ 12.07 hrs, Volume= 5,098 cf
Outflow =	=	1.50 cfs @ 12.07 hrs, Volume= 5,098 cf, Atten= 0%, Lag= 0.0 min
Primary =	=	1.50 cfs @ 12.07 hrs, Volume= 5,098 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 156.85' @ 12.10 hrs Flood Elev= 162.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	155.90'	24.0" Round Culvert L= 27.0' Ke= 0.500 Inlet / Outlet Invert= 155.90' / 155.50' S= 0.0148 '/' Cc= 0.900 n= 0.012, Flow Area= 3.14 sf

Primary OutFlow Max=1.10 cfs @ 12.07 hrs HW=156.80' TW=156.75' (Dynamic Tailwater) -1=Culvert (Outlet Controls 1.10 cfs @ 1.18 fps)

Summary for Pond 7P: DMH

 Inflow Area =
 13,317 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event

 Inflow =
 1.50 cfs @ 12.07 hrs, Volume=
 5,098 cf

 Outflow =
 1.50 cfs @ 12.07 hrs, Volume=
 5,098 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 1.50 cfs @ 12.07 hrs, Volume=
 5,098 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 157.17' @ 12.09 hrs Flood Elev= 162.77'

Device	Routing	Invert	Outlet Devices
#1	Primary	156.40'	12.0" Round Culvert L= 40.0' Ke= 0.500

> Inlet / Outlet Invert= 156.40' / 156.00' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.42 cfs @ 12.07 hrs HW=157.15' TW=156.80' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 1.42 cfs @ 3.13 fps)

Summary for Pond 8P: DMH

Inflow Area	=	8,878 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow =	=	1.00 cfs @ 12.07 hrs, Volume= 3,398 cf
Outflow =	=	1.00 cfs @ 12.07 hrs, Volume= 3,398 cf, Atten= 0%, Lag= 0.0 min
Primary =	=	1.00 cfs @ 12.07 hrs, Volume= 3,398 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 158.05' @ 12.08 hrs Flood Elev= 164.88'

Device	Routing	Invert	Outlet Devices
#1	Primary	157.50'	12.0" Round Culvert L= 154.0' Ke= 0.500
			Inlet / Outlet Invert= 157.50' / 156.50' S= 0.0065 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.99 cfs @ 12.07 hrs HW=158.05' TW=157.15' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.99 cfs @ 3.22 fps)

Summary for Pond 9P: DMH

Inflow Area =	4,439 sf,100.00% Impervious,	Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow =	0.50 cfs @ 12.07 hrs, Volume=	1,699 cf
Outflow =	0.50 cfs @ 12.07 hrs, Volume=	1,699 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.50 cfs $\overline{\textcircled{0}}$ 12.07 hrs, Volume=	1,699 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.65' @ 12.07 hrs Flood Elev= 166.83'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.30'	12.0" Round Culvert L= 141.0' Ke= 0.500 Inlet / Outlet Invert= 159.30' / 158.00' S= 0.0092 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.50 cfs @ 12.07 hrs HW=159.65' TW=158.05' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.50 cfs @ 2.02 fps)

Summary for Pond 10P: 24" Petro-Barrier

Inflow Area =		1,745 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow	=	0.20 cfs @ 12.07 hrs, Volume= 668 cf
Outflow	=	0.20 cfs @ 12.07 hrs, Volume= 668 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.20 cfs @ 12.07 hrs, Volume= 668 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.70' @ 12.07 hrs Flood Elev= 169.43'

Device	Routing	Invert	Outlet Devices
#1	Primary	162.43'	6.0" Round Culvert L= 30.0' Ke= 0.500 Inlet / Outlet Invert= 162.43' / 162.00' S= 0.0143 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.20 cfs @ 12.07 hrs HW=162.70' TW=159.65' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.20 cfs @ 1.78 fps)

Summary for Pond 11P: 24" Petro-Barrier

Inflow Area =		2,694 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow =	=	0.30 cfs @ 12.07 hrs, Volume= 1,031 cf
Outflow =	=	0.30 cfs @ 12.07 hrs, Volume= 1,031 cf, Atten= 0%, Lag= 0.0 min
Primary =	=	0.30 cfs @ 12.07 hrs, Volume= 1,031 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.98' @ 12.07 hrs Flood Elev= 168.62'

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 159.62'
 6.0" Round Culvert L= 18.0' Ke= 0.500 Inlet / Outlet Invert= 159.62' / 159.40' S= 0.0122 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.30 cfs @ 12.07 hrs HW=159.98' TW=159.65' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.30 cfs @ 2.03 fps)

Summary for Pond 12P: 24" Petro-Barrier

Inflow Area =		1,745 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow	=	0.20 cfs @ 12.07 hrs, Volume= 668 cf
Outflow	=	0.20 cfs @ 12.07 hrs, Volume= 668 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.20 cfs @ 12.07 hrs, Volume= 668 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 160.97' @ 12.07 hrs Flood Elev= 167.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	160.70'	6.0" Round Culvert L= 32.0' Ke= 0.500 Inlet / Outlet Invert= 160.70' / 160.40' S= 0.0094 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.20 cfs @ 12.07 hrs HW=160.97' TW=158.05' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.20 cfs @ 1.78 fps)

Summary for Pond 13P: 24" Petro-Barrier

Inflow Area = 2,694 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event Inflow 0.30 cfs @ 12.07 hrs. Volume= 1.031 cf = 0.30 cfs @ 12.07 hrs, Volume= Outflow = 1,031 cf, Atten= 0%, Lag= 0.0 min 0.30 cfs @ 12.07 hrs, Volume= Primary = 1.031 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 158.25' @ 12.08 hrs Flood Elev= 166.83' Device Routing Invert Outlet Devices 6.0" Round Culvert L= 20.0' Ke= 0.500 #1 157.83' Primary Inlet / Outlet Invert= 157.83' / 157.60' S= 0.0115 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.30 cfs @ 12.07 hrs HW=158.24' TW=158.05' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.30 cfs @ 2.32 fps)

Summary for Pond 14P: 24" Petro-Barrier

Inflow Area =	2,694 sf	,100.00% Impervious,	Inflow Depth = 4.59"	for NRCC 10YR 24H event
Inflow =	0.30 cfs @	12.07 hrs, Volume=	1,031 cf	
Outflow =	0.30 cfs @	12.07 hrs, Volume=	1,031 cf, Atter	n= 0%, Lag= 0.0 min
Primary =	0.30 cfs @	12.07 hrs, Volume=	1,031 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 157.32' @ 12.09 hrs Flood Elev= 165.88'

Device	Routing	Invert	Outlet Devices
#1	Primary	156.88'	6.0" Round Culvert L= 21.0' Ke= 0.500 Inlet / Outlet Invert= 156.88' / 156.60' S= 0.0133 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.28 cfs @ 12.07 hrs HW=157.31' TW=157.15' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.28 cfs @ 2.12 fps)

Summary for Pond 15P: 24" Petro-Barrier

 Inflow Area =
 1,745 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event

 Inflow =
 0.20 cfs @ 12.07 hrs, Volume=
 668 cf

 Outflow =
 0.20 cfs @ 12.07 hrs, Volume=
 668 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.20 cfs @ 12.07 hrs, Volume=
 668 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 157.94' @ 12.07 hrs Flood Elev= 164.66'

Device	Routing	Invert	Outlet Devices
#1	Primary	157.66'	6.0" Round Culvert L= 34.0' Ke= 0.500

Inlet / Outlet Invert= 157.66' / 157.40' S= 0.0076 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.20 cfs @ 12.07 hrs HW=157.94' TW=157.15' (Dynamic Tailwater)

Summary for Pond 16P: DMH

Inflow Area =	52,511 sf, 0.00% Impervious,	Inflow Depth = 1.40" for NRCC 10YR 24H event
Inflow =	1.84 cfs @ 12.10 hrs, Volume=	6,136 cf
Outflow =	1.84 cfs @ 12.10 hrs, Volume=	6,136 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.84 cfs @ 12.10 hrs, Volume=	6,136 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.74' @ 12.10 hrs Flood Elev= 162.19'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.00'	12.0" Round Culvert L= 17.0' Ke= 0.500 Inlet / Outlet Invert= 159.00' / 158.50' S= 0.0294 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.83 cfs @ 12.10 hrs HW=159.74' TW=156.79' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.83 cfs @ 2.93 fps)

Summary for Pond 17P: DMH

Inflow Area =	150,788 sf, 14.52% Impervious,	Inflow Depth = 1.87" for NRCC 10YR 24H event
Inflow =	6.20 cfs @ 12.11 hrs, Volume=	23,445 cf
Outflow =	6.20 cfs @ 12.11 hrs, Volume=	23,445 cf, Atten= 0%, Lag= 0.0 min
Primary =	6.20 cfs @ 12.11 hrs, Volume=	23,445 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 160.49' @ 12.11 hrs Flood Elev= 163.59'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.40'	24.0" Round Culvert L= 88.2' Ke= 0.500 Inlet / Outlet Invert= 159.40' / 155.50' S= 0.0442 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=6.19 cfs @ 12.11 hrs HW=160.49' TW=156.79' (Dynamic Tailwater) -1=Culvert (Inlet Controls 6.19 cfs @ 3.55 fps)

Summary for Pond 18P: DMH

Inflow Area =	128,888 sf, 0.00% Impervious,	Inflow Depth = 1.40" for NRCC 10YR 24H event
Inflow =	4.12 cfs @ 12.13 hrs, Volume=	15,062 cf
Outflow =	4.12 cfs @ 12.13 hrs, Volume=	15,062 cf, Atten= 0%, Lag= 0.0 min
Primary =	4.12 cfs @ 12.13 hrs, Volume=	15,062 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.48' @ 12.13 hrs Flood Elev= 165.39'

Device	Routing	Invert	Outlet Devices
#1	Primary	161.50'	18.0" Round Culvert L= 92.0' Ke= 0.500 Inlet / Outlet Invert= 161.50' / 159.50' S= 0.0217 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=4.12 cfs @ 12.13 hrs HW=162.48' TW=160.47' (Dynamic Tailwater) -1=Culvert (Inlet Controls 4.12 cfs @ 3.37 fps)

Summary for Pond 20P: DMH

Inflow Area	a =	102,433 sf, 12.46% Impervious, Inflow Depth = 1.72" for NRCC 10YR 24H event
Inflow	=	4.64 cfs @ 12.09 hrs, Volume= 14,644 cf
Outflow	=	4.64 cfs @ 12.09 hrs, Volume= 14,644 cf, Atten= 0%, Lag= 0.0 min
Primary	=	4.64 cfs @ 12.09 hrs, Volume= 14,644 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 152.92' @ 12.09 hrs Flood Elev= 158.03'

DeviceRoutingInvertOutlet Devices#1Primary152.00'24.0" Round Culvert L= 56.0' Ke= 0.500
Inlet / Outlet Invert= 152.00' / 150.40' S= 0.0286 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=4.63 cfs @ 12.09 hrs HW=152.92' TW=151.82' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 4.63 cfs @ 3.27 fps)

Summary for Pond 21P: DMH

Inflow Area =		25,304 sf, 16.82% Impervious, Inflow Depth = 1.83" for NRCC 10YR 24H event
Inflow	=	1.26 cfs @ 12.08 hrs, Volume= 3,869 cf
Outflow	=	1.26 cfs @ 12.08 hrs, Volume= 3,869 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.26 cfs @ 12.08 hrs, Volume= 3,869 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 155.53' @ 12.08 hrs Flood Elev= 157.94'

Device	Routing	Invert	Outlet Devices
	Primary	154.94'	12.0" Round Culvert L= 62.0' Ke= 0.500 Inlet / Outlet Invert= 154.94' / 152.40' S= 0.0410 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.26 cfs @ 12.08 hrs HW=155.53' TW=152.92' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.26 cfs @ 2.62 fps)

Summary for Pond 22P: DMH

Inflow Area = 77,129 sf, 11.04% Impervious, Inflow Depth = 1.68" for NRCC 10YR 24H event Inflow 3.38 cfs @ 12.09 hrs. Volume= 10.775 cf = 3.38 cfs @ 12.09 hrs, Volume= Outflow = 10,775 cf, Atten= 0%, Lag= 0.0 min 3.38 cfs @ 12.09 hrs, Volume= Primary = 10,775 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 156.39' @ 12.09 hrs Flood Elev= 158.66' Device Routing Invert Outlet Devices #1 155.50' 18.0" Round Culvert L= 56.0' Ke= 0.500 Primary Inlet / Outlet Invert= 155.50' / 155.00' S= 0.0089 '/' Cc= 0.900

Summary for Pond 23P: OIL/WATER SEPARATOR

n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Inflow Area =	10,110 sf,100.00% Impervious,	Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow =	1.14 cfs @ 12.07 hrs, Volume=	3,870 cf
Outflow =	1.18 cfs @ 12.07 hrs, Volume=	3,869 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.18 cfs @ 12.07 hrs, Volume=	3,869 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 152.00' @ 12.09 hrs Flood Elev= 159.18'

Device	Routing	Invert	Outlet Devices
#1	Primary	141.80'	12.0" Round Culvert L= 182.0' Ke= 0.500 Inlet / Outlet Invert= 141.80' / 140.10' S= 0.0093 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.06 cfs @ 12.07 hrs HW=151.96' TW=151.80' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 1.06 cfs @ 1.35 fps)

Summary for Pond 24P: DMH

 Inflow Area =
 10,110 sf,100.00% Impervious, Inflow Depth =
 4.59" for NRCC 10YR 24H event

 Inflow =
 1.14 cfs @
 12.07 hrs, Volume=
 3,870 cf

 Outflow =
 1.14 cfs @
 12.07 hrs, Volume=
 3,870 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 1.14 cfs @
 12.07 hrs, Volume=
 3,870 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 152.07' @ 12.10 hrs Flood Elev= 159.61'

Device	Routing	Invert	Outlet Devices
#1	Primary	150.20'	12.0" Round Culvert L= 1.0' Ke= 0.500

Inlet / Outlet Invert= 150.20' / 150.10' S= 0.1000 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.52 cfs @ 12.07 hrs HW=151.98' TW=151.96' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.52 cfs @ 0.66 fps)

Summary for Pond 25P: 24" Petro-Barrier

Inflow Area =	1,770 sf,100.00% Impervious,	Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow =	0.20 cfs @ 12.07 hrs, Volume=	678 cf
Outflow =	0.20 cfs @ 12.07 hrs, Volume=	678 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.20 cfs @ 12.07 hrs, Volume=	678 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 152.13' @ 12.11 hrs Flood Elev= 159.89'

Device	Routing	Invert	Outlet Devices
#1	Primary	150.89'	6.0" Round Culvert L= 58.0' Ke= 0.500 Inlet / Outlet Invert= 150.89' / 150.30' S= 0.0102 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.12 cfs @ 12.07 hrs HW=152.00' TW=151.98' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.12 cfs @ 0.60 fps)

Summary for Pond 26P: 24" Petro-Barrier

Inflow Area =		1,770 sf,100.00% Impervious,	Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow =		0.20 cfs @ 12.07 hrs, Volume=	678 cf
Outflow =		0.20 cfs @ 12.07 hrs, Volume=	678 cf, Atten= 0%, Lag= 0.0 min
Primary =		0.20 cfs $\overline{@}$ 12.07 hrs, Volume=	678 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.26' @ 12.07 hrs Flood Elev= 161.98'

Device	Routing	Invert	Outlet Devices
#1	Primary	152.98'	6.0" Round Culvert L= 17.0' Ke= 0.500 Inlet / Outlet Invert= 152.98' / 152.00' S= 0.0576 '/' Cc= 0.900 n= 0.010 PVC smooth interior Flow Area= 0.20 sf
			n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.20 cfs @ 12.07 hrs HW=153.26' TW=151.98' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.20 cfs @ 1.79 fps)

Summary for Pond 27P: DMH

Inflow Area =		6,570 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow	=	0.74 cfs @ 12.07 hrs, Volume= 2,515 cf
Outflow	=	0.74 cfs @ 12.07 hrs, Volume= 2,515 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.74 cfs @ 12.07 hrs, Volume= 2,515 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 154.34' @ 12.07 hrs Flood Elev= 162.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	153.90'	12.0" Round Culvert L= 224.0' Ke= 0.500 Inlet / Outlet Invert= 153.90' / 152.00' S= 0.0085 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.74 cfs @ 12.07 hrs HW=154.34' TW=151.98' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.74 cfs @ 2.25 fps)

Summary for Pond 28P: DMH

Inflow Area =		6,570 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow	=	0.74 cfs @ 12.07 hrs, Volume= 2,515 cf
Outflow	=	0.74 cfs @ 12.07 hrs, Volume= 2,515 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.74 cfs @ 12.07 hrs, Volume= 2,515 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 154.84' @ 12.07 hrs Flood Elev= 163.41'

DeviceRoutingInvertOutlet Devices#1Primary154.40'**12.0" Round Culvert** L= 50.0' Ke= 0.500
Inlet / Outlet Invert= 154.40' / 154.00' S= 0.0080 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.74 cfs @ 12.07 hrs HW=154.84' TW=154.34' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.74 cfs @ 3.22 fps)

Summary for Pond 29P: 24" Petro-Barrier

 Inflow Area =
 1,770 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event

 Inflow =
 0.20 cfs @ 12.07 hrs, Volume=
 678 cf

 Outflow =
 0.20 cfs @ 12.07 hrs, Volume=
 678 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.20 cfs @ 12.07 hrs, Volume=
 678 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 155.23' @ 12.07 hrs Flood Elev= 163.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	154.95'	6.0" Round Culvert L= 7.0' Ke= 0.500 Inlet / Outlet Invert= 154.95' / 154.50' S= 0.0643 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.20 cfs @ 12.07 hrs HW=155.23' TW=154.84' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.20 cfs @ 1.79 fps)

Summary for Pond 30P: DMH

Inflow Area = 4,800 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event Inflow 0.54 cfs @ 12.07 hrs. Volume= 1.837 cf = 0.54 cfs @ 12.07 hrs, Volume= Outflow = 1,837 cf, Atten= 0%, Lag= 0.0 min 0.54 cfs @ 12.07 hrs, Volume= Primary = 1.837 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 160.77' @ 12.07 hrs Flood Elev= 165.34' Device Routing Invert Outlet Devices #1 160.40' 12.0" Round Culvert L= 98.0' Ke= 0.500 Primary Inlet / Outlet Invert= 160.40' / 159.50' S= 0.0092 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.54 cfs @ 12.07 hrs HW=160.77' TW=154.84' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 0.54 cfs @ 2.06 fps)

Summary for Pond 31P: DMH

 Inflow Area =
 4,800 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event

 Inflow =
 0.54 cfs @ 12.07 hrs, Volume=
 1,837 cf

 Outflow =
 0.54 cfs @ 12.07 hrs, Volume=
 1,837 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.54 cfs @ 12.07 hrs, Volume=
 1,837 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.27' @ 12.07 hrs Flood Elev= 168.53'

Device	Routing	Invert	Outlet Devices
#1	Primary	161.90'	12.0" Round Culvert L= 232.0' Ke= 0.500
			Inlet / Outlet Invert= 161.90' / 160.50' S= 0.0060 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.54 cfs @ 12.07 hrs HW=162.27' TW=160.77' (Dynamic Tailwater) -1=Culvert (Barrel Controls 0.54 cfs @ 2.98 fps)

Summary for Pond 32P: 24" Petro-Barrier

 Inflow Area =
 1,600 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event

 Inflow =
 0.18 cfs @ 12.07 hrs, Volume=
 612 cf

 Outflow =
 0.18 cfs @ 12.07 hrs, Volume=
 612 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.18 cfs @ 12.07 hrs, Volume=
 612 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.70' @ 12.07 hrs Flood Elev= 170.69'

Device	Routing	Invert	Outlet Devices
#1	Primary	162.44'	6.0" Round Culvert L= 10.0' Ke= 0.500

Inlet / Outlet Invert= 162.44' / 162.20' S= 0.0240 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.18 cfs @ 12.07 hrs HW=162.70' TW=162.27' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.18 cfs @ 1.74 fps)

Summary for Pond 33P: DMH

Inflow Area =	3,200 sf,100.00% Impervious,	Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow =	0.36 cfs @ 12.07 hrs, Volume=	1,225 cf
Outflow =	0.36 cfs @ 12.07 hrs, Volume=	1,225 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.36 cfs @ 12.07 hrs, Volume=	1,225 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 163.50' @ 12.07 hrs Flood Elev= 170.43'

Device	Routing	Invert	Outlet Devices
#1	Primary	163.20'	12.0" Round Culvert L= 159.0' Ke= 0.500 Inlet / Outlet Invert= 163.20' / 162.00' S= 0.0075 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.36 cfs @ 12.07 hrs HW=163.50' TW=162.27' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.36 cfs @ 1.85 fps)

Summary for Pond 34P: 24" Petro-Barrier

Inflow Area =		1,600 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow	=	0.18 cfs @ 12.07 hrs, Volume= 612 cf
Outflow	=	0.18 cfs @ 12.07 hrs, Volume= 612 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.18 cfs @ 12.07 hrs, Volume= 612 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 163.69' @ 12.07 hrs Flood Elev= 171.68'

Device	Routing	Invert	Outlet Devices
#1	Primary	163.43'	6.0" Round Culvert L= 8.0' Ke= 0.500 Inlet / Outlet Invert= 163.43' / 163.30' S= 0.0162 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.18 cfs @ 12.07 hrs HW=163.69' TW=163.50' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.18 cfs @ 2.53 fps)

Summary for Pond 35P: 24" Petro-Barrier

Inflow Area =		1,600 sf,100.00% Impervious, Inflow Depth = 4.59" for NRCC 10YR 24H event
Inflow	=	0.18 cfs @ 12.07 hrs, Volume= 612 cf
Outflow	=	0.18 cfs @ 12.07 hrs, Volume= 612 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.18 cfs @ 12.07 hrs, Volume= 612 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 164.65' @ 12.07 hrs Flood Elev= 172.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	164.39'	6.0" Round Culvert L= 38.0' Ke= 0.500 Inlet / Outlet Invert= 164.39' / 164.00' S= 0.0103 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.18 cfs @ 12.07 hrs HW=164.65' TW=163.50' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.18 cfs @ 1.74 fps)

Summary for Pond 40P: DMH

Inflow Area	=	105,170 sf, 0.00% Impervious, Inflow Depth = 0.85" for NRCC 10YR 24H event
Inflow	=	1.35 cfs @ 12.09 hrs, Volume= 7,429 cf
Outflow	=	1.35 cfs @ 12.09 hrs, Volume= 7,429 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.35 cfs @ 12.09 hrs, Volume= 7,429 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 158.40' @ 12.12 hrs Flood Elev= 160.03'

Device	Routing	Invert	Outlet Devices
#1	Primary	158.00'	24.0" W x 12.0" H Box Culvert L= 10.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 158.00' / 157.70' S= 0.0300 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 2.00 sf

Primary OutFlow Max=1.31 cfs @ 12.09 hrs HW=158.40' TW=158.24' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.31 cfs @ 1.65 fps)

Summary for Pond 41P: CULVERT

Inflow Area =	87,839 sf, 0.00% Impervious,	Inflow Depth = 0.51" for NRCC 10YR 24H event
Inflow =	0.53 cfs @ 12.27 hrs, Volume=	3,720 cf
Outflow =	0.53 cfs @ 12.27 hrs, Volume=	3,720 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.53 cfs @ 12.27 hrs, Volume=	3,720 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.19' @ 12.27 hrs Flood Elev= 161.08'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.00'	24.0" W x 12.0" H Box Culvert L= 55.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 159.00' / 158.00' S= 0.0182 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 2.00 sf

Primary OutFlow Max=0.53 cfs @ 12.27 hrs HW=159.19' TW=158.37' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.53 cfs @ 1.40 fps)

Summary for Link 1L: OVERFLOW

Inflow Are	a =	782,870 sf,	7.42% Impervious,	Inflow Depth = 0.00"	for NRCC 10YR 24H event
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Summary for Link 10L: OFF-SITE

Inflow Area =	258,650 sf, 0.00% Impervious,	Inflow Depth = 0.02" for NRCC 10YR 24H event
Inflow =	0.01 cfs @ 22.05 hrs, Volume=	332 cf
Primary =	0.01 cfs @ 22.05 hrs, Volume=	332 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: AREA OUTSIDE	Runoff Area=258,650 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=32 Runoff=0.00 cfs 0 cf
Subcatchment 2S: AREA OUTSIDE	Runoff Area=285,632 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=491' Tc=12.3 min CN=31 Runoff=0.00 cfs 0 cf
Subcatchment 3S: S SWALE 1	Runoff Area=21,901 sf 0.00% Impervious Runoff Depth=0.31"
Flow Length=378	Slope=0.0200 '/' Tc=6.2 min CN=56 Runoff=0.07 cfs 563 cf
Subcatchment 4S: S SWALE 2	Runoff Area=37,407 sf 0.00% Impervious Runoff Depth=0.01" Tc=5.0 min CN=41 Runoff=0.00 cfs 36 cf
Subcatchment 5S: S SWALE 3	Runoff Area=28,531 sf 0.00% Impervious Runoff Depth=0.11"
Flow Length=644	' Slope=0.0200 '/' Tc=9.3 min CN=48 Runoff=0.01 cfs 251 cf
Subcatchment 6S: ACCESS RAMP	Runoff Area=17,331 sf 0.00% Impervious Runoff Depth=1.34" Tc=5.0 min CN=78 Runoff=0.64 cfs 1,935 cf
Subcatchment 7S: S SWALE 4	Runoff Area=20,291 sf 0.00% Impervious Runoff Depth=0.56"
Flow Length=644	' Slope=0.0200 '/' Tc=9.3 min CN=63 Runoff=0.20 cfs 947 cf
Subcatchment 8S: N SWALE 1	Runoff Area=15,778 sf 0.00% Impervious Runoff Depth=1.47"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=80 Runoff=0.62 cfs 1,935 cf
Subcatchment 9S: N SWALE 2	Runoff Area=14,101 sf 0.00% Impervious Runoff Depth=1.68"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=83 Runoff=0.63 cfs 1,978 cf
Subcatchment 10S: N SWALE 3	Runoff Area=12,739 sf 0.00% Impervious Runoff Depth=1.68"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=83 Runoff=0.57 cfs 1,787 cf
Subcatchment 11S: NW PERF. DRAIN	Runoff Area=45,853 sf 0.00% Impervious Runoff Depth=0.56"
Flow Length=660'	Slope=0.0200 '/' Tc=6.0 min CN=63 Runoff=0.51 cfs 2,140 cf
Subcatchment 12S: CENTRAL PERF. DRAII	N Runoff Area=6,658 sf 0.00% Impervious Runoff Depth=0.56" Tc=5.0 min CN=63 Runoff=0.08 cfs 311 cf
Subcatchment 13S: GIS BUILDINGS	Runoff Area=21,900 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=1.66 cfs 5,579 cf
Subcatchment 14S: GIS BLDGS PERF	Runoff Area=74,801 sf 0.00% Impervious Runoff Depth=0.56"
Flow Length=570'	Slope=0.0200 '/' Tc=8.4 min CN=63 Runoff=0.76 cfs 3,492 cf
	N Runoff Area=54,087 sf 0.00% Impervious Runoff Depth=0.56" Slope=0.0200 '/' Tc=8.4 min CN=63 Runoff=0.55 cfs 2,525 cf
Subcatchment 16S: N PERF. DRAIN	Runoff Area=35,583 sf 0.57% Impervious Runoff Depth=0.56" Tc=5.0 min CN=63 Runoff=0.41 cfs 1,661 cf

CWW Substation 5-Parcel Proposed ConditioType III 24-hr NRCC 2YR 24H Rainfall=3.29"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 78			
Subcatchment17S: SE PERF. DRAIN 1 Flow Length=425'	Runoff Area=41,546 sf 20.00% Impervious Runoff Depth=0.88" Slope=0.0200 '/' Tc=5.9 min CN=70 Runoff=0.89 cfs 3,050 cf		
Subcatchment 18S: SE PERF. DRAIN 2	Runoff Area=25,304 sf 16.82% Impervious Runoff Depth=0.83" Tc=5.0 min CN=69 Runoff=0.52 cfs 1,752 cf		
Subcatchment20S: SR4	Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.13 cfs 445 cf		
Subcatchment 21S: XFMR 1	Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.20 cfs 686 cf		
Subcatchment22S: SR5	Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.13 cfs 445 cf		
Subcatchment 23S: XFMR 2	Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.20 cfs 686 cf		
Subcatchment 24S: XFMR 3	Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.20 cfs 686 cf		
Subcatchment25S: SR6	Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.13 cfs 445 cf		
Subcatchment 26S: SR1	Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.12 cfs 408 cf		
Subcatchment 27S: SR2	Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.12 cfs 408 cf		
Subcatchment 28S: SR3	Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.12 cfs 408 cf		
Subcatchment 29S: STATCOM 1	Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.13 cfs 451 cf		
Subcatchment 30S: STATCOM 2	Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.13 cfs 451 cf		
Subcatchment31S: STATCOM3	Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=3.06" Tc=5.0 min CN=98 Runoff=0.13 cfs 451 cf		
	Avg. Flow Depth=0.15' Max Vel=1.00 fps Inflow=0.07 cfs 563 cf 309.0' S=0.0137 '/' Capacity=10.56 cfs Outflow=0.07 cfs 563 cf		
	Avg. Flow Depth=0.14' Max Vel=1.14 fps Inflow=0.07 cfs 599 cf 257.3' S=0.0195 '/' Capacity=12.62 cfs Outflow=0.07 cfs 599 cf		
	Avg. Flow Depth=0.15' Max Vel=1.01 fps Inflow=0.07 cfs 850 cf 329.0' S=0.0142 '/' Capacity=10.77 cfs Outflow=0.07 cfs 850 cf		
	vg. Flow Depth=0.40' Max Vel=1.40 fps Inflow=0.78 cfs 3,732 cf 67.8' S=0.0073 '/' Capacity=9.98 cfs Outflow=0.68 cfs 3,732 cf		

CWW Substation 5-Parcel Proposed ConditioType III 24-hr	NRCC 2YR 24H Rainfall=3.29"
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HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLC	Page 79

Reach 5R: DRAINAGE CHANN n=(EL Avg. Flow Depth=0.13' Max Vel=2.65 fps Inflow=6.76 cfs 26,478 cf 0.078 L=175.0' S=0.3086 '/' Capacity=198.63 cfs Outflow=6.70 cfs 26,478 cf
Reach 11R: N SWALE PART 1	Avg. Flow Depth=0.32' Max Vel=1.75 fps Inflow=0.62 cfs 1,935 cf n=0.030 L=447.0' S=0.0153 '/' Capacity=11.19 cfs Outflow=0.54 cfs 1,935 cf
Reach 12R: N SWALE PART 2	Avg. Flow Depth=0.42' Max Vel=1.97 fps Inflow=1.13 cfs 3,913 cf n=0.030 L=431.0' S=0.0137 '/' Capacity=10.59 cfs Outflow=1.03 cfs 3,913 cf
Reach 13R: N SWALE PART 3	Avg. Flow Depth=0.48' Max Vel=2.11 fps Inflow=1.53 cfs 5,700 cf n=0.030 L=386.0' S=0.0131 '/' Capacity=10.33 cfs Outflow=1.45 cfs 5,700 cf
	LOCALIZED Peak Elev=86.09' Storage=12,721 cf Inflow=6.70 cfs 26,478 cf scarded=0.47 cfs 26,478 cf Primary=0.00 cfs 0 cf Outflow=0.47 cfs 26,478 cf
Pond 2P: SEDIMENT FOREBA	Y Peak Elev=148.32' Storage=4,904 cf Inflow=2.11 cfs 9,431 cf Discarded=0.12 cfs 9,433 cf Primary=0.00 cfs 0 cf Outflow=0.12 cfs 9,433 cf
Pond 3P: DMHs w/ Vortex Unit	Peak Elev=148.98' Inflow=6.76 cfs 26,478 cf 36.0" Round Culvert n=0.011 L=25.0' S=0.1200 '/' Outflow=6.76 cfs 26,478 cf
Pond 4P: DMH	Peak Elev=151.28' Inflow=6.76 cfs 26,478 cf 36.0" Round Culvert n=0.011 L=36.0' S=0.0639 '/' Outflow=6.76 cfs 26,478 cf
Pond 5P: DMH 24	Peak Elev=156.27' Inflow=4.21 cfs 17,440 cf 4.0" Round Culvert n=0.011 L=263.0' S=0.0186 '/' Outflow=4.21 cfs 17,440 cf
Pond 6P: OIL/WATER SEPARA	ATOR Peak Elev=156.43' Inflow=1.01 cfs 3,393 cf 24.0" Round Culvert n=0.012 L=27.0' S=0.0148 '/' Outflow=1.01 cfs 3,393 cf
Pond 7P: DMH	Peak Elev=156.92' Inflow=1.01 cfs 3,393 cf 12.0" Round Culvert n=0.011 L=40.0' S=0.0100 '/' Outflow=1.01 cfs 3,393 cf
Pond 8P: DMH	Peak Elev=157.93' Inflow=0.67 cfs 2,262 cf 12.0" Round Culvert n=0.011 L=154.0' S=0.0065 '/' Outflow=0.67 cfs 2,262 cf
Pond 9P: DMH	Peak Elev=159.59' Inflow=0.34 cfs 1,131 cf 12.0" Round Culvert n=0.011 L=141.0' S=0.0092 '/' Outflow=0.34 cfs 1,131 cf
Pond 10P: 24" Petro-Barrier	Peak Elev=162.65' Inflow=0.13 cfs 445 cf 6.0" Round Culvert n=0.010 L=30.0' S=0.0143 '/' Outflow=0.13 cfs 445 cf
Pond 11P: 24" Petro-Barrier	Peak Elev=159.90' Inflow=0.20 cfs 686 cf 6.0" Round Culvert n=0.010 L=18.0' S=0.0122 '/' Outflow=0.20 cfs 686 cf
Pond 12P: 24" Petro-Barrier	Peak Elev=160.92' Inflow=0.13 cfs 445 cf 6.0" Round Culvert n=0.010 L=32.0' S=0.0094 '/' Outflow=0.13 cfs 445 cf
Pond 13P: 24" Petro-Barrier	Peak Elev=158.14' Inflow=0.20 cfs 686 cf 6.0" Round Culvert n=0.010 L=20.0' S=0.0115 '/' Outflow=0.20 cfs 686 cf

CWW Substation 5-Parcel Proposed ConditioType III 24-hr NRCC 2YR 24H Rainfall=3.29" Prepared by Stantec Consulting Ltd. Printed 2/10/2023 HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLC Page 80 Peak Elev=157.17' Inflow=0.20 cfs 686 cf Pond 14P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=21.0' S=0.0133 '/' Outflow=0.20 cfs 686 cf Peak Elev=157.88' Inflow=0.13 cfs 445 cf Pond 15P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=34.0' S=0.0076 '/' Outflow=0.13 cfs 445 cf Peak Elev=159.38' Inflow=0.58 cfs 2,451 cf Pond 16P: DMH 12.0" Round Culvert n=0.011 L=17.0' S=0.0294 '/' Outflow=0.58 cfs 2,451 cf Peak Elev=160.08' Inflow=2.68 cfs 11,596 cf Pond 17P: DMH 24.0" Round Culvert n=0.011 L=88.2' S=0.0442 '/' Outflow=2.68 cfs 11,596 cf Peak Elev=162.01' Inflow=1.30 cfs 6,017 cf Pond 18P: DMH 18.0" Round Culvert n=0.011 L=92.0' S=0.0217 '/' Outflow=1.30 cfs 6,017 cf Pond 20P: DMH Peak Elev=152.56' Inflow=1.82 cfs 6,463 cf 24.0" Round Culvert n=0.011 L=56.0' S=0.0286 '/' Outflow=1.82 cfs 6,463 cf Peak Elev=155.30' Inflow=0.52 cfs 1,752 cf Pond 21P: DMH 12.0" Round Culvert n=0.011 L=62.0' S=0.0410 '/' Outflow=0.52 cfs 1,752 cf Pond 22P: DMH Peak Elev=156.01' Inflow=1.30 cfs 4,711 cf 18.0" Round Culvert n=0.011 L=56.0' S=0.0089 '/' Outflow=1.30 cfs 4,711 cf Peak Elev=151.35' Inflow=0.78 cfs 2,576 cf Pond 23P: OIL/WATER SEPARATOR 12.0" Round Culvert n=0.011 L=182.0' S=0.0093 '/' Outflow=0.77 cfs 2,575 cf Pond 24P: DMH Peak Elev=151.39' Inflow=0.77 cfs 2,576 cf 12.0" Round Culvert n=0.011 L=1.0' S=0.1000 '/' Outflow=0.78 cfs 2,576 cf Peak Elev=151.42' Inflow=0.13 cfs 451 cf Pond 25P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=58.0' S=0.0102 '/' Outflow=0.13 cfs 451 cf Pond 26P: 24" Petro-Barrier Peak Elev=153.20' Inflow=0.13 cfs 451 cf 6.0" Round Culvert n=0.010 L=17.0' S=0.0576 '/' Outflow=0.13 cfs 451 cf Peak Elev=154.25' Inflow=0.50 cfs 1,674 cf Pond 27P: DMH 12.0" Round Culvert n=0.011 L=224.0' S=0.0085 '/' Outflow=0.50 cfs 1,674 cf Pond 28P: DMH Peak Elev=154.76' Inflow=0.50 cfs 1,674 cf 12.0" Round Culvert n=0.011 L=50.0' S=0.0080 '/' Outflow=0.50 cfs 1,674 cf Peak Elev=155.17' Inflow=0.13 cfs 451 cf Pond 29P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=7.0' S=0.0643 '/' Outflow=0.13 cfs 451 cf Peak Elev=160.70' Inflow=0.36 cfs 1,223 cf Pond 30P: DMH 12.0" Round Culvert n=0.011 L=98.0' S=0.0092 '/' Outflow=0.36 cfs 1,223 cf Peak Elev=162.21' Inflow=0.36 cfs 1,223 cf Pond 31P: DMH 12.0" Round Culvert n=0.011 L=232.0' S=0.0060 '/' Outflow=0.36 cfs 1,223 cf Peak Elev=162.65' Inflow=0.12 cfs 408 cf Pond 32P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=10.0' S=0.0240 '/' Outflow=0.12 cfs 408 cf

Pond 33P: DMH	Peak Elev=163.44' Inflow=0.24 cfs 815 cf 12.0" Round Culvert n=0.011 L=159.0' S=0.0075 '/' Outflow=0.24 cfs 815 cf
Pond 34P: 24" Petro-Barrier	Peak Elev=163.64' Inflow=0.12 cfs 408 cf 6.0" Round Culvert n=0.010 L=8.0' S=0.0162 '/' Outflow=0.12 cfs 408 cf
Pond 35P: 24" Petro-Barrier	Peak Elev=164.60' Inflow=0.12 cfs 408 cf 6.0" Round Culvert n=0.010 L=38.0' S=0.0103 '/' Outflow=0.12 cfs 408 cf
Pond 40P: DMH 24	Peak Elev=158.22' Inflow=0.64 cfs 2,784 cf 4.0" x 12.0" Box Culvert n=0.011 L=10.0' S=0.0300 '/' Outflow=0.64 cfs 2,784 cf
Pond 41P: CULVERT	Peak Elev=159.05' Inflow=0.07 cfs 850 cf 24.0" x 12.0" Box Culvert n=0.011 L=55.0' S=0.0182 '/' Outflow=0.07 cfs 850 cf
Link 1L: OVERFLOW	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Link 10L: OFF-SITE	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

Total Runoff Area = 1,041,520 sf Runoff Volume = 35,910 cf Average Runoff Depth = 0.41" 94.42% Pervious = 983,426 sf 5.58% Impervious = 58,094 sf

Summary for Subcatchment 1S: AREA OUTSIDE SUBSTATION - NOT TO POND

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

A	rea (sf)	CN	Description			
1	79,269	30	Woods, Go	od, HSG A		
	72,994	30	Meadow, non-grazed, HSG A			
	6,387	96	Gravel surfa	ace, HSG A	λ	
2	58,650	32	Weighted Average			
2	58,650		100.00% Pe	ervious Are	a	
Тс	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)		
5.0					Direct Entry, Direct Entry	

Summary for Subcatchment 2S: AREA OUTSIDE SUBSTATION - TO POND

Runoff	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Depth= 0.00"
--------	---	------------	-------------------	--------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

A	rea (sf)	CN [Description		
2	206,991	30 V	Voods, Go	od, HSG A	
	72,294	30 N	Meadow, no	on-grazed,	HSG A
	6,347	96 (Gravel surfa	ace, HSG A	Ι
2	285,632 31 Weighted Average			verage	
2	85,632	1	100.00% Pe	ervious Are	a
Тс	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.4	50	0.5000	0.25		Sheet Flow, Sheet
					Woods: Light underbrush n= 0.400 P2= 3.29"
8.9	441	0.1100	0.83		Shallow Concentrated Flow, Shallow Conc
					Forest w/Heavy Litter Kv= 2.5 fps
12.3	491	Total			

Summary for Subcatchment 3S: S SWALE 1

Runoff = 0.07 cfs @ 12.29 hrs, Volume= 563 cf, Depth= 0.31"

_	A	rea (sf)	CN	Description				
*		3,599	63	63 Crushed Stone Surface, HSG A				
		6,746	96	Gravel surfa	ace, HSG A	A		
		7,202		Woods, Go	,			
_		4,354	30	Meadow, no	on-grazed,	HSG A		
		21,901		Weighted A				
		21,901		100.00% Pe	ervious Are	a		
	Тс	Length	Slope		Capacity	Description		
	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)			
	2.3	50	0.0200	0.36		Sheet Flow, Sheet		
						Fallow n= 0.050 P2= 3.29"		
	3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc		
_						Nearly Bare & Untilled Kv= 10.0 fps		
	6.2	378	Total					
				Summar	y for Sub	catchment 4S: S SWALE 2		
					-			

Runoff = 0.00 cfs @ 21.85 hrs, Volume=

36 cf, Depth= 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

/	Area (sf)	CN	Description				
	5,160	96	Gravel surfa	ace, HSG A	A		
*	2,578	63	Crushed Stone Surface, HSG A				
	20,025	30	Woods, Goo	Woods, Good, HSG A			
	9,644	30	Meadow, no	Meadow, non-grazed, HSG A			
	37,407	41	Weighted A	verage			
	37,407		100.00% Pe	ervious Are	а		
Tc (min)		Slop (ft/fl		Capacity (cfs)	Description		
5.0					Direct Entry, Direct Entry		

Summary for Subcatchment 5S: S SWALE 3

Runoff = 0.01 cfs @ 13.77 hrs, Volume= 251 cf, Depth= 0.11"

	Area (sf)	CN	Description
*	3,335	63	Crushed Stone Surface, HSG A
	7,061	30	Woods, Good, HSG A
	6,315	96	Gravel surface, HSG A
	11,820	30	Meadow, non-grazed, HSG A
	28,531	48	Weighted Average
	28,531		100.00% Pervious Area

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
						Fallow n= 0.050 P2= 3.29"
	7.0	594	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps
-						Nearly Date & Onlined INV- 10.0 1p3
	9.3	644	Total			

Summary for Subcatchment 6S: ACCESS RAMP

Runoff = 0.64 cfs @ 12.08 hrs, Volume= 1,935 cf, Depth= 1.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

Area (sf)	CN	Description				
12,617	96	Gravel surfa	ace, HSG A	A Contraction of the second seco		
1,094	30	Woods, Go	od, HSG A			
3,620	30	Meadow, no	on-grazed,	HSG A		
17,331	78	78 Weighted Average				
17,331		100.00% Pe	ervious Are	а		
Tc Length	ı Slop	be Velocity	Capacity	Description		
(min) (feet)) (ft/	ft) (ft/sec)	(cfs)			
5.0				Direct Entry, Direct Entry		
				••••••		
17,331 17,331 Tc Length (min) (feet)	78 n Slop	Weighted A 100.00% Pe be Velocity	verage ervious Are Capacity	a		

Summary for Subcatchment 7S: S SWALE 4

Runoff = 0.20 cfs @ 12.16 hrs, Volume= 947 cf, Depth= 0.56"

_	А	rea (sf)	CN [Description					
*		6,230	63 (
		7,051	96 (Gravel surfa	ravel surface, HSG A				
_		7,010	30 I	Aeadow, no	eadow, non-grazed, HSG A				
		20,291	63 \	0 0					
		20,291		100.00% Pervious Area					
	Tc	Length	Slope		Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	2.3	50	0.0200	0.36		Sheet Flow, Sheet			
						Fallow n= 0.050 P2= 3.29"			
	7.0	594	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc			
						Nearly Bare & Untilled Kv= 10.0 fps			
	9.3	644	Total						

Summary for Subcatchment 8S: N SWALE 1

Runoff = 0.62 cfs @ 12.09 hrs, Volume= 1,935 cf, Depth= 1.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

_	A	rea (sf)	CN	Description					
*		4,949	63	Crushed Stone Surface, HSG A					
		9,543	96	Gravel surfa	ace, HSG A	A Í			
		1,286	30	Meadow, no	leadow, non-grazed, HSG A				
		15,778	80	0 Weighted Average					
		15,778		100.00% Pervious Area					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	2.3	50	0.0200	0.36		Sheet Flow, Sheet			
						Fallow n= 0.050 P2= 3.29"			
	3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc			
_						Nearly Bare & Untilled Kv= 10.0 fps			
	6.0	270	Total						

6.2 378 Total

Summary for Subcatchment 9S: N SWALE 2

Runoff = 0.63 cfs @ 12.09 hrs, Volume= 1,978 cf, Depth= 1.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

_	A	rea (sf)	CN [Description		
*		5,475	63 (Crushed St	one Surfac	e, HSG A
_		8,626	96 (Gravel surfa	ace, HSG A	λ
		14,101	83 \	Neighted A	verage	
14,101 100.00% Pervious Area					ervious Are	а
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
	3.9	328	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps
_		070	T ()			

6.2 378 Total

Summary for Subcatchment 10S: N SWALE 3

Runoff = 0.57 cfs @ 12.09 hrs, Volume= 1,787 cf, Depth= 1.68"

	A	rea (sf)	CN I	Description		
*		4,958	63 (Crushed St	one Surfac	e, HSG A
_		7,781	96 (Gravel surfa	ace, HSG A	Ι
		12,739	83 \	Neighted A	verage	
		12,739		100.00% Pe	ervious Are	а
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
						Fallow n= 0.050 P2= 3.29"
	3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc
						Nearly Bare & Untilled Kv= 10.0 fps
_	6.2	378	Total			

Summary for Subcatchment 11S: NW PERF. DRAIN

Runoff = 0.51 cfs @ 12.11 hrs, Volume= 2,140 cf, Depth= 0.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

_	A	rea (sf)	CN E	Description		
*		45,853	63 C	Crushed St	one Surface	e, HSG A
		45,853	1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
	3.0	255	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps
	0.7	355	0.0200	8.34	6.55	Pipe Channel, Perf Pipe
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
_	6.0	660	Total			

Summary for Subcatchment 12S: CENTRAL PERF. DRAIN 2

Runoff = 0.08 cfs @ 12.10 hrs, Volume= 311 cf, Depth= 0.56"

	Area (sf)	CN	Description	
*	6,658	63	Crushed Stone Surface, HSG A	
	6,658		100.00% Pervious Area	

Prepare	CWW Substation 5-Parcel Proposed ConditioType III 24-hr NRCC 2YR 24H Rainfall=3.29"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 87						
Tc (min)	Tc Length Slope Velocity Capacity Description						
5.0					Direct Entry, Direct Entry		
		Su	mmary fo	or Subca	tchment 13S: GIS BUILDINGS		
Runoff	=	1.66 cf	s @ 12.0	7 hrs, Volu	ume= 5,579 cf, Depth= 3.06"		
			nod, UH=S 24H Rainf		nted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs		
A	rea (sf)	CN E	escription				
	21,900		Roofs, HSC				
	21,900	1	00.00% In	npervious A	Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry, Direct Entry		
	:	Summa	ry for Su	ıbcatchm	nent 14S: GIS BLDGS PERF DRAIN		
Runoff	=	0.76 cf	s@ 12.1	5 hrs, Volu	ume= 3,492 cf, Depth= 0.56"		
			nod, UH=S 24H Rainf		nted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs		
A	rea (sf)	CN E	escription				
	74,801			one Surfac			
	74,801	1	00.00% Pe	ervious Are	a		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
2.3	50	0.0200	0.36		Sheet Flow, Sheet		
6.1	520	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps		
8.4	570	Total					
	9	Summa	ry for Su	bcatchm	ent 15S: CENTRAL PERF. DRAIN 1		
Runoff	=	0.55 cf	s @ 12.1	5 hrs, Volu	ume= 2,525 cf, Depth= 0.56"		
Runoff h		2_20 meti	nod UH-9	CS Woidh	ted_{CN} Time Span= 0.00-72.00 brs. dt= 0.01 brs		

_	Area (sf)	CN	Description
*	54,087	63	Crushed Stone Surface, HSG A
	54,087		100.00% Pervious Area

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
	6.1	520	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc
	0.1	520	0.0200	1.41		Nearly Bare & Untilled Kv= 10.0 fps
_	8.4	570	Total			

Summary for Subcatchment 16S: N PERF. DRAIN

Runoff = 0.41 cfs @ 12.10 hrs, Volume= 1,661 cf, Depth= 0.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

	A	rea (sf)	CN	Description					
*		35,381 202	63 98	Crushed Stone Surface, HSG A Roofs, HSG A					
		35,583 35,381 202	63	Weighted A 99.43% Per 0.57% Impe	vious Area				
	Tc (min)	Length (feet)	Slop (ft/ff	,	Capacity (cfs)	Description			
_	5.0					Direct Entry, Direct Entry			

Summary for Subcatchment 17S: SE PERF. DRAIN 1

Runoff = 0.89 cfs @ 12.10 hrs, Volume= 3,050 cf, Depth= 0.88"

	A	rea (sf)	CN [Description		
		8,310	98 F	Roofs, HSC	βA	
*		33,236	63 (Crushed St	one Surface	e, HSG A
		41,546	70 \	Neighted A	verage	
		33,236	8	30.00% Pei	vious Area	
		8,310	2	20.00% Imp	pervious Are	ea
	Тс	Length	Slope		Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
						Fallow n= 0.050 P2= 3.29"
	3.5	300	0.0200	1.41		Shallow Concentrated Flow, Shallow
						Nearly Bare & Untilled Kv= 10.0 fps
	0.1	75	0.0200	8.34	6.55	Pipe Channel, Perf. Pipe
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
						n= 0.010 PVC, smooth interior
	5.9	425	Total			

Summary for Subcatchment 18S: SE PERF. DRAIN 2

Runoff = 0.52 cfs @ 12.09 hrs, Volume= 1,752 cf, Depth= 0.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

	A	rea (sf)	CN	Description		
		4,255	98	Roofs, HSC	βA	
*		21,049	63	Crushed St	one Surfac	e, HSG A
		25,304 21,049 4,255	69	Weighted A 83.18% Per 16.82% Imp	vious Area pervious Ar	ea
	Тс	Length	Slope		Capacity	Description
((min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	5.0					Direct Entry, Direct Entry

Summary for Subcatchment 20S: SR4

Runoff = 0.13 cfs @ 12.07 hrs, Volume= 445 cf, Depth= 3.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

A	rea (sf)	CN I	Description					
	1,745	98	Concrete C	ontainment				
	1,745		100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry, Direct Entry			

Summary for Subcatchment 21S: XFMR 1

Runoff = 0.20 cfs @ 12.07 hrs, Volume= 686 cf, Depth= 3.06"

A	rea (sf)	CN E	Description					
	2,694	98 (Concrete Containment					
	2,694	1	100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry, Direct Entry			

Summary for Subcatchment 22S: SR5

Runoff = 0.13 cfs @ 12.07 hrs, Volume= 445 cf, Depth= 3.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

A	rea (sf)	CN	Descriptio	n			
	1,745	98	Concrete	Containmen	t		
	1,745 100.00% Impervious Area						
Tc (min)	Length (feet)	Slop (ft/f			Description		
5.0		•	, , , , , , , , , , , , , , , , , , ,		Direct Entry,	Direct E	Intry
					•		-
Summary for Subcatchment 23S: XFMR 2							
Runoff	=	0.20	cfs @ 12	07 hrs, Volu	ume=	686 cf,	Depth= 3.06"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"							
A	rea (sf)	CN	Descriptio	n			
	2,694	98	Concrete	<u>Containmen</u>	t		
	2,694		100.00%	mpervious A	Area		
Tc (min)	Length (feet)	Slop (ft/f			Description		
5.0 Direct Entry, Direct Entry							
Summary for Subcatchment 24S: XFMR 3							
Runoff	=	0.20	cfs @ 12	07 hrs, Volu	ume=	686 cf,	Depth= 3.06"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"							
A	rea (sf)	CN	Descriptio				
	2,694	98	Concrete	Containmen	t		
	2,694		100.00%	mpervious A	Area		

Description

Direct Entry, Direct Entry

(cfs)

Slope Velocity Capacity

(ft/sec)

Tc

(min)

5.0

Length

(feet)

(ft/ft)

Summary for Subcatchment 25S: SR6

Runoff = 0.13 cfs @ 12.07 hrs, Volume= 445 cf, Depth= 3.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

A	rea (sf)	CN I	Description				
	1,745	98	Concrete Co	ontainment			
	1,745 100.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
5.0					Direct Entry,	, Direct Entry	
Summary for Subcatchment 26S: SR1							
Runoff	=	0.12 c	fs @ 12.0	7 hrs, Volu	ime=	408 cf, Depth= 3.06"	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"							
A	rea (sf)	CN I	Description				
	1,600	98	Concrete Co	ontainment			
	1,600		100.00% Im	pervious A	rea		
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
5.0 Direct Entry, Direct Entry							
Summary for Subcatchment 27S: SR2							
Runoff	=	0.12 c	fs @ 12.0	7 hrs, Volu	ime=	408 cf, Depth= 3.06"	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"							
Area (sf) CN Description							
	1,600		Concrete Co				
	1,600		100.00% Im	pervious A	rea		
	Length		Velocity		Description		

(ft/ft)

(min)

5.0

(feet)

(ft/sec)

(cfs)

Direct Entry, Direct Entry

Summary for Subcatchment 28S: SR3

Runoff = 0.12 cfs @ 12.07 hrs, Volume= 408 cf, Depth= 3.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"

Area (sf) CN Description							
1,600 98 Concrete Containment							
1,600 100.00% Impervious Area							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
5.0 Direct Entry, Direct Entry							
Summary for Subcatchment 29S: STATCOM 1 TRANSFORMER							
Runoff = 0.13 cfs @ 12.07 hrs, Volume= 451 cf, Depth= 3.06"							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"							
Area (sf) CN Description							
1,770 98 Concrete Containment							
1,770 100.00% Impervious Area							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
5.0 Direct Entry, Direct Entry							
Summary for Subcatchment 30S: STATCOM 2 TRANSFORMER							
Runoff = 0.13 cfs @ 12.07 hrs, Volume= 451 cf, Depth= 3.06"							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr NRCC 2YR 24H Rainfall=3.29"							
Area (sf) CN Description							
1,770 98 Concrete Containment							
1,770 100.00% Impervious Area							
Tc Length Slope Velocity Capacity Description							

(feet)

(min)

5.0

(ft/ft)

(ft/sec)

(cfs)

Direct Entry, Direct Entry

Summary for Subcatchment 31S: STATCOM 3 TRANSFORMER

Runoff = 0.13 cfs @ 12.07 hrs, Volume= 451 cf, Depth= 3.06"

Area (sf) CN Description							
1,770 98 Concrete Containment							
1,770 100.00% Impervious Area							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
5.0 Direct Entry, Direct Entry							
Summary for Reach 1R: S SWALE PART 1							
Inflow Area = 21,901 sf, 0.00% Impervious, Inflow Depth = 0.31" for NRCC 2YR 24H event Inflow = 0.07 cfs @ 12.29 hrs, Volume= 563 cf Outflow = 0.07 cfs @ 12.37 hrs, Volume= 563 cf, Atten= 4%, Lag= 4.3 min							
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 1.00 fps, Min. Travel Time= 5.1 min Avg. Velocity = 0.58 fps, Avg. Travel Time= 8.8 min							
Peak Storage= 21 cf @ 12.37 hrs Average Depth at Peak Storage= 0.15' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 38.49 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.56 cfs							
0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 309.0' Slope= 0.0137 '/' Inlet Invert= 174.13', Outlet Invert= 169.91'							

Summary for Reach 2R: S SWALE PART 2

Inflow Are	ea =	59,308 sf, 0.00% Impervious, Inflow	Depth = 0.12" for NRCC 2YR 24H event
Inflow	=	0.07 cfs @ 12.37 hrs, Volume=	599 cf
Outflow	=	0.07 cfs @ 12.42 hrs, Volume=	599 cf, Atten= 2%, Lag= 3.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 1.14 fps, Min. Travel Time= 3.8 min Avg. Velocity = 0.67 fps, Avg. Travel Time= 6.4 min

Peak Storage= 15 cf @ 12.42 hrs Average Depth at Peak Storage= 0.14' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 46.01 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 12.62 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 257.3' Slope= 0.0195 '/' Inlet Invert= 169.90', Outlet Invert= 164.88'

Summary for Reach 3R: S SWALE PART 3

Inflow Are	a =	87,839 sf,	0.00% Impervious,	Inflow Depth = 0.12 "	for NRCC 2YR 24H event
Inflow	=	0.07 cfs @ 12	2.46 hrs, Volume=	850 cf	
Outflow	=	0.07 cfs @ 12	2.53 hrs, Volume=	850 cf, Atte	n= 4%, Lag= 4.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 1.01 fps, Min. Travel Time= 5.4 min Avg. Velocity = 0.64 fps, Avg. Travel Time= 8.6 min

Peak Storage= 22 cf @ 12.53 hrs Average Depth at Peak Storage= 0.15' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 39.24 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.77 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 329.0' Slope= 0.0142 '/' Inlet Invert= 164.80', Outlet Invert= 160.13'

Summary for Reach 4R: S SWALE PART 4

Inflow Area = 125,461 sf, 0.00% Impervious, Inflow Depth = 0.36" for NRCC 2YR 24H event 0.78 cfs @ 12.09 hrs. Volume= Inflow 3.732 cf = 0.68 cfs @ 12.14 hrs, Volume= Outflow = 3,732 cf, Atten= 13%, Lag= 3.2 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 1.40 fps, Min. Travel Time= 4.4 min Avg. Velocity = 0.61 fps, Avg. Travel Time= 10.1 min Peak Storage= 177 cf @ 12.14 hrs Average Depth at Peak Storage= 0.40' Defined Flood Depth= 2.00' Flow Area= 9.5 sf, Capacity= 31.59 cfs Bank-Full Depth= 1.10' Flow Area= 3.6 sf, Capacity= 9.98 cfs 0.00' x 1.10' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value = 3.0 '/' Top Width = 6.60' Length= 367.8' Slope= 0.0073 '/' Inlet Invert= 157.70', Outlet Invert= 155.00'

Summary for Reach 5R: DRAINAGE CHANNEL

 Inflow Area =
 497,238 sf, 11.68% Impervious, Inflow Depth = 0.64" for NRCC 2YR 24H event

 Inflow =
 6.76 cfs @ 12.09 hrs, Volume=
 26,478 cf

 Outflow =
 6.70 cfs @ 12.10 hrs, Volume=
 26,478 cf, Atten= 1%, Lag= 0.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.65 fps, Min. Travel Time= 1.1 min Avg. Velocity = 0.70 fps, Avg. Travel Time= 4.2 min

Peak Storage= 443 cf @ 12.10 hrs Average Depth at Peak Storage= 0.13' Defined Flood Depth= 1.00' Flow Area= 20.0 sf, Capacity= 198.63 cfs Bank-Full Depth= 1.00' Flow Area= 20.0 sf, Capacity= 198.63 cfs

20.00' x 1.00' deep channel, n= 0.078 Riprap, 12-inch Length= 175.0' Slope= 0.3086 '/' Inlet Invert= 145.00', Outlet Invert= 91.00'

Summary for Reach 11R: N SWALE PART 1

Inflow Area = 15,778 sf, 0.00% Impervious, Inflow Depth = 1.47" for NRCC 2YR 24H event Inflow 0.62 cfs @ 12.09 hrs. Volume= 1.935 cf = 0.54 cfs @ 12.14 hrs, Volume= Outflow = 1,935 cf, Atten= 13%, Lag= 2.8 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 1.75 fps, Min. Travel Time= 4.3 min Avg. Velocity = 0.71 fps, Avg. Travel Time= 10.5 min Peak Storage= 138 cf @ 12.14 hrs Average Depth at Peak Storage= 0.32' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 40.78 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 11.19 cfs 0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 447.0' Slope= 0.0153 '/' Inlet Invert= 174.03', Outlet Invert= 167.18'

Summary for Reach 12R: N SWALE PART 2

 Inflow Area =
 29,879 sf,
 0.00% Impervious,
 Inflow Depth =
 1.57"
 for
 NRCC 2YR 24H event

 Inflow =
 1.13 cfs @
 12.11 hrs,
 Volume=
 3,913 cf

 Outflow =
 1.03 cfs @
 12.16 hrs,
 Volume=
 3,913 cf,
 Atten= 9%,
 Lag= 2.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 1.97 fps, Min. Travel Time= 3.6 min Avg. Velocity = 0.76 fps, Avg. Travel Time= 9.5 min

Peak Storage= 226 cf @ 12.16 hrs Average Depth at Peak Storage= 0.42' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 38.60 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.59 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 431.0' Slope= 0.0137 '/' Inlet Invert= 167.10', Outlet Invert= 161.18' CWW Substation 5-Parcel Proposed ConditioType III 24-hr NRCC 2YR 24H Rainfall=3.29" Prepared by Stantec Consulting Ltd. Printed 2/10/2023 HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLC Page 97 Summary for Reach 13R: N SWALE PART 3 Inflow Area = 42,618 sf, 0.00% Impervious, Inflow Depth = 1.60" for NRCC 2YR 24H event Inflow 1.53 cfs @ 12.13 hrs. Volume= 5.700 cf = Outflow = 1.45 cfs @ 12.17 hrs, Volume= 5,700 cf, Atten= 5%, Lag= 2.4 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.11 fps, Min. Travel Time= 3.1 min Avg. Velocity = 0.79 fps, Avg. Travel Time= 8.1 min Peak Storage= 265 cf @ 12.17 hrs Average Depth at Peak Storage= 0.48' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 37.64 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.33 cfs 0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 386.0' Slope= 0.0131 '/' Inlet Invert= 161.10', Outlet Invert= 156.06' Summary for Pond 1P: DETENTION AREA - LOCALIZED DEPRESSION NE Inflow Area = 782,870 sf, 7.42% Impervious, Inflow Depth = 0.41" for NRCC 2YR 24H event 6.70 cfs @ 12.10 hrs, Volume= Inflow 26,478 cf = 0.47 cfs @ 15.13 hrs, Volume= Outflow 26,478 cf, Atten= 93%, Lag= 181.7 min = 26,478 cf Discarded = 0.47 cfs @ 15.13 hrs, Volume= 0.00 hrs. Volume= Primarv 0.00 cfs @ 0 cf= Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 86.09' @ 15.13 hrs Surf.Area= 8,395 sf Storage= 12,721 cf Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 351.3 min(1,187.3 - 836.0)

Volume	Invert	Avail.Storage	Storage Description
#1	83.00'	125,742 cf	Custom Stage Data (Irregular)Listed below (Recalc)

CWW Substation 5-Parcel Proposed Conditio*Type III 24-hr NRCC 2YR 24H Rainfall=3.29"* Prepared by Stantec Consulting Ltd. Printed 2/10/2023

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Page 98

Wet.Area (sq-ft)	Cum.Store (cubic-feet)	Inc.Store (cubic-feet)	Perim. (feet)	Surf.Area (sq-ft)	Elevation (feet)
827	0	0	135.0	827	83.00
3,131	1,698	1,698	217.0	2,756	84.00
6,460	5,528	3,830	298.0	5,016	85.00
11,079	12,004	6,477	383.0	8,057	86.00
18,370	22,130	10,126	488.0	12,347	87.00
27,043	37,200	15,070	589.0	17,968	88.00
37,342	58,633	21,433	690.0	25,096	89.00
59,011	88,001	29,368	865.0	33,858	90.00
69,242	125,742	37,740	936.1	41,761	91.00

Device	Routing	Invert	Outlet Devices
#1	Discarded	83.00'	2.410 in/hr Exfiltration over Surface area
#2	Primary	90.60'	50.0' long x 4.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66
			2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32

Discarded OutFlow Max=0.47 cfs @ 15.13 hrs HW=86.09' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.47 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=83.00' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Pond 2P: SEDIMENT FOREBAY

Inflow Area =	168,079 sf, 0.00% Impervious,	Inflow Depth = 0.67" for NRCC 2YR 24H event
Inflow =	2.11 cfs @ 12.16 hrs, Volume=	9,431 cf
Outflow =	0.12 cfs @ 16.35 hrs, Volume=	9,433 cf, Atten= 94%, Lag= 251.0 min
Discarded =	0.12 cfs @ 16.35 hrs, Volume=	9,433 cf
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 148.32' @ 16.35 hrs Surf.Area= 2,238 sf Storage= 4,904 cf Flood Elev= 151.00' Surf.Area= 2,531 sf Storage= 11,301 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 423.8 min (1,292.8 - 868.9)

Volume	Invert	Avail.Sto	rage Storage	Description		
#1	146.00'	22,34	48 cf Custom	i Stage Data (Cor	nic) Listed below (Recalc)
Elevatic (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
146.0 155.0	-	2,000 3,000	0 22,348	0 22,348	2,000 3,876	
Device	Routing	Invert	Outlet Device	S		
#1 #2	Discarded Primary	146.00' 150.00'		xfiltration over S 2.0' breadth Broa		angular Weir

> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.12 cfs @ 16.35 hrs HW=148.32' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=146.00' TW=145.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 3P: DMHs w/ Vortex Unit

 Inflow Area =
 329,159 sf, 17.65% Impervious, Inflow Depth = 0.97" for NRCC 2YR 24H event

 Inflow =
 6.76 cfs @ 12.09 hrs, Volume=
 26,478 cf

 Outflow =
 6.76 cfs @ 12.09 hrs, Volume=
 26,478 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 6.76 cfs @ 12.09 hrs, Volume=
 26,478 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 148.98' @ 12.09 hrs Flood Elev= 151.05'

DeviceRoutingInvertOutlet Devices#1Primary148.00'**36.0" Round Culvert** L= 25.0' Ke= 0.500
Inlet / Outlet Invert= 148.00' / 145.00' S= 0.1200 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf

Primary OutFlow Max=6.75 cfs @ 12.09 hrs HW=148.98' TW=145.13' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 6.75 cfs @ 3.37 fps)

Summary for Pond 4P: DMH

 Inflow Area =
 329,159 sf, 17.65% Impervious, Inflow Depth = 0.97" for NRCC 2YR 24H event

 Inflow =
 6.76 cfs @ 12.09 hrs, Volume=
 26,478 cf

 Outflow =
 6.76 cfs @ 12.09 hrs, Volume=
 26,478 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 6.76 cfs @ 12.09 hrs, Volume=
 26,478 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 151.28' @ 12.09 hrs Flood Elev= 158.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	150.30'	36.0" Round Culvert L= 36.0' Ke= 0.500 Inlet / Outlet Invert= 150.30' / 148.00' S= 0.0639 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf

Primary OutFlow Max=6.75 cfs @ 12.09 hrs HW=151.28' TW=148.98' (Dynamic Tailwater) -1=Culvert (Inlet Controls 6.75 cfs @ 3.37 fps)

Summary for Pond 5P: DMH

Inflow Area = 216,616 sf, 16.26% Impervious, Inflow Depth = 0.97" for NRCC 2YR 24H event Inflow 4.21 cfs @ 12.09 hrs. Volume= 17.440 cf = 4.21 cfs @ 12.09 hrs, Volume= Outflow = 17,440 cf, Atten= 0%, Lag= 0.0 min 4.21 cfs @ 12.09 hrs, Volume= Primary = 17,440 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 156.27' @ 12.09 hrs Flood Elev= 161.58'

DeviceRoutingInvertOutlet Devices#1Primary155.40'24.0" Round Culvert L= 263.0' Ke= 0.500
Inlet / Outlet Invert= 155.40' / 150.50' S= 0.0186 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=4.20 cfs @ 12.09 hrs HW=156.27' TW=151.28' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 4.20 cfs @ 3.18 fps)

Summary for Pond 6P: OIL/WATER SEPARATOR

Inflow Area =	13,317 sf,100.00% Impervious,	Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow =	1.01 cfs @ 12.07 hrs, Volume=	3,393 cf
Outflow =	1.01 cfs @ 12.07 hrs, Volume=	3,393 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.01 cfs @ 12.07 hrs, Volume=	3,393 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 156.43' @ 12.09 hrs Flood Elev= 162.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	155.90'	24.0" Round Culvert L= 27.0' Ke= 0.500 Inlet / Outlet Invert= 155.90' / 155.50' S= 0.0148 '/' Cc= 0.900 n= 0.012, Flow Area= 3.14 sf

Primary OutFlow Max=0.96 cfs @ 12.07 hrs HW=156.43' TW=156.26' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.96 cfs @ 2.19 fps)

Summary for Pond 7P: DMH

 Inflow Area =
 13,317 sf,100.00% Impervious, Inflow Depth =
 3.06" for NRCC 2YR 24H event

 Inflow =
 1.01 cfs @
 12.07 hrs, Volume=
 3,393 cf

 Outflow =
 1.01 cfs @
 12.07 hrs, Volume=
 3,393 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 1.01 cfs @
 12.07 hrs, Volume=
 3,393 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 156.92' @ 12.08 hrs Flood Elev= 162.77'

Device	Routing	Invert	Outlet Devices
#1	Primary	156.40'	12.0" Round Culvert L= 40.0' Ke= 0.500

Inlet / Outlet Invert= 156.40' / 156.00' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.00 cfs @ 12.07 hrs HW=156.92' TW=156.43' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 1.00 cfs @ 3.49 fps)

Summary for Pond 8P: DMH

Inflow Area =	8,878 sf,100.00% Impervious,	Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow =	0.67 cfs @ 12.07 hrs, Volume=	2,262 cf
Outflow =	0.67 cfs @ 12.07 hrs, Volume=	2,262 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.67 cfs @ 12.07 hrs, Volume=	2,262 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 157.93' @ 12.07 hrs Flood Elev= 164.88'

DeviceRoutingInvertOutlet Devices#1Primary157.50'**12.0" Round Culvert** L= 154.0' Ke= 0.500
Inlet / Outlet Invert= 157.50' / 156.50' S= 0.0065 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.67 cfs @ 12.07 hrs HW=157.93' TW=156.92' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.67 cfs @ 3.08 fps)

Summary for Pond 9P: DMH

Inflow Area =	4,439 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow =	0.34 cfs @ 12.07 hrs, Volume= 1,131 cf
Outflow =	0.34 cfs @ 12.07 hrs, Volume= 1,131 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.34 cfs @ 12.07 hrs, Volume= 1,131 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.59' @ 12.07 hrs Flood Elev= 166.83'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.30'	12.0" Round Culvert L= 141.0' Ke= 0.500 Inlet / Outlet Invert= 159.30' / 158.00' S= 0.0092 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.34 cfs @ 12.07 hrs HW=159.59' TW=157.93' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.34 cfs @ 1.82 fps)

Summary for Pond 10P: 24" Petro-Barrier

Inflow Area =		1,745 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow	=	0.13 cfs @ 12.07 hrs, Volume= 445 cf
Outflow	=	0.13 cfs @ 12.07 hrs, Volume= 445 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.13 cfs @ 12.07 hrs, Volume= 445 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.65' @ 12.07 hrs Flood Elev= 169.43'

Device	Routing	Invert	Outlet Devices
#1	Primary	162.43'	6.0" Round Culvert L= 30.0' Ke= 0.500 Inlet / Outlet Invert= 162.43' / 162.00' S= 0.0143 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.13 cfs @ 12.07 hrs HW=162.65' TW=159.59' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.13 cfs @ 1.60 fps)

Summary for Pond 11P: 24" Petro-Barrier

Inflow Area =		2,694 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow	=	0.20 cfs @ 12.07 hrs, Volume= 686 cf
Outflow	=	0.20 cfs @ 12.07 hrs, Volume= 686 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.20 cfs @ 12.07 hrs, Volume= 686 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.90' @ 12.07 hrs Flood Elev= 168.62'

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 159.62'
 6.0" Round Culvert L= 18.0' Ke= 0.500 Inlet / Outlet Invert= 159.62' / 159.40' S= 0.0122 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.20 cfs @ 12.07 hrs HW=159.90' TW=159.59' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.20 cfs @ 1.80 fps)

Summary for Pond 12P: 24" Petro-Barrier

Inflow Area =		1,745 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow	=	0.13 cfs @ 12.07 hrs, Volume= 445 cf
Outflow	=	0.13 cfs @ 12.07 hrs, Volume= 445 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.13 cfs @ 12.07 hrs, Volume= 445 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 160.92' @ 12.07 hrs Flood Elev= 167.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	160.70'	6.0" Round Culvert L= 32.0' Ke= 0.500 Inlet / Outlet Invert= 160.70' / 160.40' S= 0.0094 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.13 cfs @ 12.07 hrs HW=160.92' TW=157.93' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.13 cfs @ 1.60 fps)

Summary for Pond 13P: 24" Petro-Barrier

Inflow Area = 2,694 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event Inflow 0.20 cfs @ 12.07 hrs, Volume= 686 cf = 0.20 cfs @ 12.07 hrs, Volume= Outflow = 686 cf, Atten= 0%, Lag= 0.0 min 0.20 cfs @ 12.07 hrs, Volume= Primary = 686 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 158.14' @ 12.07 hrs Flood Elev= 166.83' Device Routing Invert Outlet Devices

 00100	rtouting	Invort	Callet Devices
 #1	Primary	157.83'	6.0" Round Culvert L= 20.0' Ke= 0.500
	-		Inlet / Outlet Invert= 157.83' / 157.60' S= 0.0115 '/' Cc= 0.900
			n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.20 cfs @ 12.07 hrs HW=158.14' TW=157.93' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.20 cfs @ 2.31 fps)

Summary for Pond 14P: 24" Petro-Barrier

Inflow Area =		2,694 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow	=	0.20 cfs @ 12.07 hrs, Volume= 686 cf
Outflow	=	0.20 cfs @ 12.07 hrs, Volume= 686 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.20 cfs @ 12.07 hrs, Volume= 686 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 157.17' @ 12.08 hrs Flood Elev= 165.88'

Device	Routing	Invert	Outlet Devices
#1	Primary	156.88'	6.0" Round Culvert L= 21.0' Ke= 0.500 Inlet / Outlet Invert= 156.88' / 156.60' S= 0.0133 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.20 cfs @ 12.07 hrs HW=157.17' TW=156.92' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.20 cfs @ 2.47 fps)

Summary for Pond 15P: 24" Petro-Barrier

 Inflow Area =
 1,745 sf,100.00% Impervious, Inflow Depth =
 3.06"
 for NRCC 2YR 24H event

 Inflow =
 0.13 cfs @
 12.07 hrs, Volume=
 445 cf

 Outflow =
 0.13 cfs @
 12.07 hrs, Volume=
 445 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.13 cfs @
 12.07 hrs, Volume=
 445 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 157.88' @ 12.07 hrs Flood Elev= 164.66'

Device	Routing	Invert	Outlet Devices
#1	Primary	157.66'	6.0" Round Culvert L= 34.0' Ke= 0.500

Inlet / Outlet Invert= 157.66' / 157.40' S= 0.0076 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.13 cfs @ 12.07 hrs HW=157.88' TW=156.92' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 0.13 cfs @ 2.30 fps)

Summary for Pond 16P: DMH

Inflow Area =	52,511 sf, 0.00% Impervious,	Inflow Depth = 0.56" for NRCC 2YR 24H event
Inflow =	0.58 cfs @ 12.11 hrs, Volume=	2,451 cf
Outflow =	0.58 cfs @ 12.11 hrs, Volume=	2,451 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.58 cfs @ 12.11 hrs, Volume=	2,451 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.38' @ 12.11 hrs Flood Elev= 162.19'

DeviceRoutingInvertOutlet Devices#1Primary159.00'**12.0" Round Culvert** L= 17.0' Ke= 0.500
Inlet / Outlet Invert= 159.00' / 158.50' S= 0.0294 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.58 cfs @ 12.11 hrs HW=159.38' TW=156.27' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.58 cfs @ 2.11 fps)

Summary for Pond 17P: DMH

Inflow Area =	150,788 sf, 14.52% Impervious,	Inflow Depth = 0.92" for NRCC 2YR 24H event
Inflow =	2.68 cfs @ 12.10 hrs, Volume=	11,596 cf
Outflow =	2.68 cfs @ 12.10 hrs, Volume=	11,596 cf, Atten= 0%, Lag= 0.0 min
Primary =	2.68 cfs @ 12.10 hrs, Volume=	11,596 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 160.08' @ 12.10 hrs Flood Elev= 163.59'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.40'	24.0" Round Culvert L= 88.2' Ke= 0.500 Inlet / Outlet Invert= 159.40' / 155.50' S= 0.0442 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=2.67 cfs @ 12.10 hrs HW=160.08' TW=156.27' (Dynamic Tailwater) -1=Culvert (Inlet Controls 2.67 cfs @ 2.82 fps)

Summary for Pond 18P: DMH

Inflow Area =	128,888 sf, 0.00% Impervious,	Inflow Depth = 0.56" for NRCC 2YR 24H event
Inflow =	1.30 cfs @ 12.15 hrs, Volume=	6,017 cf
Outflow =	1.30 cfs @ 12.15 hrs, Volume=	6,017 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.30 cfs @ 12.15 hrs, Volume=	6,017 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.01' @ 12.15 hrs Flood Elev= 165.39'

Device	Routing	Invert	Outlet Devices
#1	Primary	161.50'	18.0" Round Culvert L= 92.0' Ke= 0.500 Inlet / Outlet Invert= 161.50' / 159.50' S= 0.0217 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=1.30 cfs @ 12.15 hrs HW=162.01' TW=160.05' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.30 cfs @ 2.44 fps)

Summary for Pond 20P: DMH

Inflow Area =	=	102,433 sf, 12.46% Impervious, Inflow Depth = 0.76" for NRCC 2YR 24H event
Inflow =	:	1.82 cfs @ 12.09 hrs, Volume= 6,463 cf
Outflow =	:	1.82 cfs @ 12.09 hrs, Volume= 6,463 cf, Atten= 0%, Lag= 0.0 min
Primary =	:	1.82 cfs @ 12.09 hrs, Volume= 6,463 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 152.56' @ 12.09 hrs Flood Elev= 158.03'

DeviceRoutingInvertOutlet Devices#1Primary152.00'24.0" Round CulvertL= 56.0'Ke= 0.500Inlet / Outlet Invert=152.00' / 150.40'S= 0.0286 '/'Cc= 0.900n=0.011Concrete pipe, straight & clean, Flow Area=3.14 sf

Primary OutFlow Max=1.82 cfs @ 12.09 hrs HW=152.56' TW=151.28' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.82 cfs @ 2.54 fps)

Summary for Pond 21P: DMH

Inflow Are	ea =	25,304 sf, 16.82% Impervious, Inflow Depth = 0.83" for NRCC 2YR 24H event
Inflow	=	0.52 cfs @ 12.09 hrs, Volume= 1,752 cf
Outflow	=	0.52 cfs @ 12.09 hrs, Volume= 1,752 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.52 cfs @ 12.09 hrs, Volume= 1,752 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 155.30' @ 12.09 hrs Flood Elev= 157.94'

Device	Routing	Invert	Outlet Devices
	Primary	154.94'	12.0" Round Culvert L= 62.0' Ke= 0.500 Inlet / Outlet Invert= 154.94' / 152.40' S= 0.0410 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.52 cfs @ 12.09 hrs HW=155.30' TW=152.56' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.52 cfs @ 2.04 fps)

Summary for Pond 22P: DMH

Inflow Area = 77,129 sf, 11.04% Impervious, Inflow Depth = 0.73" for NRCC 2YR 24H event Inflow 1.30 cfs @ 12.10 hrs. Volume= 4.711 cf = 1.30 cfs @ 12.10 hrs, Volume= Outflow = 4,711 cf, Atten= 0%, Lag= 0.0 min 1.30 cfs @ 12.10 hrs, Volume= Primary = 4,711 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 156.01' @ 12.10 hrs Flood Elev= 158.66' Device Routing Invert Outlet Devices #1 155.50' 18.0" Round Culvert L= 56.0' Ke= 0.500 Primary Inlet / Outlet Invert= 155.50' / 155.00' S= 0.0089 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=1.30 cfs @ 12.10 hrs HW=156.01' TW=152.56' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.30 cfs @ 2.44 fps)

Summary for Pond 23P: OIL/WATER SEPARATOR

Inflow Area	=	10,110 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow	=	0.78 cfs @ 12.07 hrs, Volume= 2,576 cf
Outflow	=	0.77 cfs @ 12.08 hrs, Volume= 2,575 cf, Atten= 2%, Lag= 0.6 min
Primary	=	0.77 cfs @ 12.08 hrs, Volume= 2,575 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 151.35' @ 12.10 hrs Flood Elev= 159.18'

Device	Routing	Invert	Outlet Devices
#1	Primary	141.80'	12.0" Round Culvert L= 182.0' Ke= 0.500 Inlet / Outlet Invert= 141.80' / 140.10' S= 0.0093 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.70 cfs @ 12.08 hrs HW=151.35' TW=151.28' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.70 cfs @ 0.90 fps)

Summary for Pond 24P: DMH

 Inflow Area =
 10,110 sf,100.00% Impervious, Inflow Depth =
 3.06" for NRCC 2YR 24H event

 Inflow =
 0.77 cfs @
 12.07 hrs, Volume=
 2,576 cf

 Outflow =
 0.78 cfs @
 12.07 hrs, Volume=
 2,576 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.78 cfs @
 12.07 hrs, Volume=
 2,576 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 151.39' @ 12.10 hrs Flood Elev= 159.61'

Device	Routing	Invert	Outlet Devices
#1	Primary	150.20'	12.0" Round Culvert L= 1.0' Ke= 0.500

Inlet / Outlet Invert= 150.20' / 150.10' S= 0.1000 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.45 cfs @ 12.07 hrs HW=151.34' TW=151.32' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.45 cfs @ 0.57 fps)

Summary for Pond 25P: 24" Petro-Barrier

Inflow Area =	1,770	sf,100.00% Impervious	Inflow Depth = 3.06" for NRCC 2YR 24H event	
Inflow =	0.13 cfs (12.07 hrs, Volume=	451 cf	
Outflow =	0.13 cfs (12.07 hrs, Volume=	451 cf, Atten= 0%, Lag= 0.0 min	
Primary =	0.13 cfs () 12.07 hrs, Volume=	451 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 151.42' @ 12.10 hrs Flood Elev= 159.89'

Device	Routing	Invert	Outlet Devices
#1	Primary	150.89'	6.0" Round Culvert L= 58.0' Ke= 0.500 Inlet / Outlet Invert= 150.89' / 150.30' S= 0.0102 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.05 cfs @ 12.07 hrs HW=151.34' TW=151.34' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.05 cfs @ 0.32 fps)

Summary for Pond 26P: 24" Petro-Barrier

Inflow Area =	1,770 sf,100.00% Impervious,	Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow =	0.13 cfs @ 12.07 hrs, Volume=	451 cf
Outflow =	0.13 cfs @ 12.07 hrs, Volume=	451 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.13 cfs @ 12.07 hrs, Volume=	451 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.20' @ 12.07 hrs Flood Elev= 161.98'

Device	Routing	Invert	Outlet Devices
#1	Primary	152.98'	6.0" Round Culvert L= 17.0' Ke= 0.500 Inlet / Outlet Invert= 152.98' / 152.00' S= 0.0576 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.13 cfs @ 12.07 hrs HW=153.20' TW=151.34' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.13 cfs @ 1.60 fps)

Summary for Pond 27P: DMH

Inflow Area =		6,570 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow	=	0.50 cfs @ 12.07 hrs, Volume= 1,674 cf
Outflow	=	0.50 cfs @ 12.07 hrs, Volume= 1,674 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.50 cfs @ 12.07 hrs, Volume= 1,674 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 154.25' @ 12.07 hrs Flood Elev= 162.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	153.90'	12.0" Round Culvert L= 224.0' Ke= 0.500 Inlet / Outlet Invert= 153.90' / 152.00' S= 0.0085 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.50 cfs @ 12.07 hrs HW=154.25' TW=151.34' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.50 cfs @ 2.02 fps)

Summary for Pond 28P: DMH

Inflow Area	a =	6,570 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow	=	0.50 cfs @ 12.07 hrs, Volume= 1,674 cf
Outflow	=	0.50 cfs @ 12.07 hrs, Volume= 1,674 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.50 cfs @ 12.07 hrs, Volume= 1,674 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 154.76' @ 12.07 hrs Flood Elev= 163.41'

DeviceRoutingInvertOutlet Devices#1Primary154.40'**12.0" Round Culvert** L= 50.0' Ke= 0.500
Inlet / Outlet Invert= 154.40' / 154.00' S= 0.0080 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.50 cfs @ 12.07 hrs HW=154.76' TW=154.25' (Dynamic Tailwater) -1=Culvert (Barrel Controls 0.50 cfs @ 2.95 fps)

Summary for Pond 29P: 24" Petro-Barrier

 Inflow Area =
 1,770 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event

 Inflow =
 0.13 cfs @ 12.07 hrs, Volume=
 451 cf

 Outflow =
 0.13 cfs @ 12.07 hrs, Volume=
 451 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.13 cfs @ 12.07 hrs, Volume=
 451 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 155.17' @ 12.07 hrs Flood Elev= 163.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	154.95'	6.0" Round Culvert L= 7.0' Ke= 0.500 Inlet / Outlet Invert= 154.95' / 154.50' S= 0.0643 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.13 cfs @ 12.07 hrs HW=155.17' TW=154.76' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.13 cfs @ 1.60 fps)

Summary for Pond 30P: DMH

Inflow Area = 4,800 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event Inflow 0.36 cfs @ 12.07 hrs. Volume= 1.223 cf = 0.36 cfs @ 12.07 hrs, Volume= Outflow = 1,223 cf, Atten= 0%, Lag= 0.0 min 0.36 cfs @ 12.07 hrs, Volume= Primary = 1,223 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 160.70' @ 12.07 hrs Flood Elev= 165.34' Device Routing Invert Outlet Devices #1 160.40' 12.0" Round Culvert L= 98.0' Ke= 0.500 Primary Inlet / Outlet Invert= 160.40' / 159.50' S= 0.0092 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.36 cfs @ 12.07 hrs HW=160.70' TW=154.76' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 0.36 cfs @ 1.86 fps)

Summary for Pond 31P: DMH

 Inflow Area =
 4,800 sf,100.00% Impervious, Inflow Depth =
 3.06" for NRCC 2YR 24H event

 Inflow =
 0.36 cfs @
 12.07 hrs, Volume=
 1,223 cf

 Outflow =
 0.36 cfs @
 12.07 hrs, Volume=
 1,223 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.36 cfs @
 12.07 hrs, Volume=
 1,223 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.21' @ 12.07 hrs Flood Elev= 168.53'

Device	Routing	Invert	Outlet Devices
#1	Primary	161.90'	12.0" Round Culvert L= 232.0' Ke= 0.500 Inlet / Outlet Invert= 161.90' / 160.50' S= 0.0060 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.36 cfs @ 12.07 hrs HW=162.21' TW=160.70' (Dynamic Tailwater) -1=Culvert (Barrel Controls 0.36 cfs @ 2.68 fps)

Summary for Pond 32P: 24" Petro-Barrier

 Inflow Area =
 1,600 sf,100.00% Impervious, Inflow Depth =
 3.06" for NRCC 2YR 24H event

 Inflow =
 0.12 cfs @
 12.07 hrs, Volume=
 408 cf

 Outflow =
 0.12 cfs @
 12.07 hrs, Volume=
 408 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.12 cfs @
 12.07 hrs, Volume=
 408 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.65' @ 12.07 hrs Flood Elev= 170.69'

Device	Routing	Invert	Outlet Devices
#1	Primary	162.44'	6.0" Round Culvert L= 10.0' Ke= 0.500

Inlet / Outlet Invert= 162.44' / 162.20' S= 0.0240 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.12 cfs @ 12.07 hrs HW=162.65' TW=162.21' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.12 cfs @ 1.56 fps)

Summary for Pond 33P: DMH

Inflow Area =	=	3,200 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow =	=	0.24 cfs @ 12.07 hrs, Volume= 815 cf
Outflow =	=	0.24 cfs @ 12.07 hrs, Volume= 815 cf, Atten= 0%, Lag= 0.0 min
Primary =	=	0.24 cfs @ 12.07 hrs, Volume= 815 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 163.44' @ 12.07 hrs Flood Elev= 170.43'

Device	Routing	Invert	Outlet Devices
#1	Primary	163.20'	12.0" Round Culvert L= 159.0' Ke= 0.500 Inlet / Outlet Invert= 163.20' / 162.00' S= 0.0075 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.24 cfs @ 12.07 hrs HW=163.44' TW=162.21' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.24 cfs @ 1.67 fps)

Summary for Pond 34P: 24" Petro-Barrier

Inflow Area	=	1,600 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow =	=	0.12 cfs @ 12.07 hrs, Volume= 408 cf
Outflow =	=	0.12 cfs @ 12.07 hrs, Volume= 408 cf, Atten= 0%, Lag= 0.0 min
Primary =	=	0.12 cfs @ 12.07 hrs, Volume= 408 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 163.64' @ 12.07 hrs Flood Elev= 171.68'

Device	Routing	Invert	Outlet Devices
#1	Primary	163.43'	6.0" Round Culvert L= 8.0' Ke= 0.500 Inlet / Outlet Invert= 163.43' / 163.30' S= 0.0162 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.12 cfs @ 12.07 hrs HW=163.64' TW=163.44' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.12 cfs @ 1.56 fps)

Summary for Pond 35P: 24" Petro-Barrier

Inflow Are	a =	1,600 sf,100.00% Impervious, Inflow Depth = 3.06" for NRCC 2YR 24H event
Inflow	=	0.12 cfs @ 12.07 hrs, Volume= 408 cf
Outflow	=	0.12 cfs @ 12.07 hrs, Volume= 408 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.12 cfs @ 12.07 hrs, Volume= 408 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 164.60' @ 12.07 hrs Flood Elev= 172.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	164.39'	6.0" Round Culvert L= 38.0' Ke= 0.500 Inlet / Outlet Invert= 164.39' / 164.00' S= 0.0103 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.12 cfs @ 12.07 hrs HW=164.60' TW=163.44' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.12 cfs @ 1.56 fps)

Summary for Pond 40P: DMH

Inflow Area =	=	105,170 sf, 0.00% Impervious, Inflow Depth = 0.32" for NRCC 2YR 24H event
Inflow =		0.64 cfs @ 12.08 hrs, Volume= 2,784 cf
Outflow =		0.64 cfs @ 12.08 hrs, Volume= 2,784 cf, Atten= 0%, Lag= 0.0 min
Primary =		0.64 cfs @ 12.08 hrs, Volume= 2,784 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 158.22' @ 12.09 hrs Flood Elev= 160.03'

Device	Routing	Invert	Outlet Devices
#1	Primary	158.00'	24.0" W x 12.0" H Box Culvert L= 10.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 158.00' / 157.70' S= 0.0300 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 2.00 sf

Primary OutFlow Max=0.63 cfs @ 12.08 hrs HW=158.22' TW=158.07' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.63 cfs @ 1.44 fps)

Summary for Pond 41P: CULVERT

Inflow Area =	87,839 sf, 0.00% Impervious,	Inflow Depth = 0.12" for NRCC 2YR 24H event
Inflow =	0.07 cfs @ 12.53 hrs, Volume=	850 cf
Outflow =	0.07 cfs @ 12.53 hrs, Volume=	850 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.07 cfs @ 12.53 hrs, Volume=	850 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.05' @ 12.53 hrs Flood Elev= 161.08'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.00'	24.0" W x 12.0" H Box Culvert L= 55.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 159.00' / 158.00' S= 0.0182 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 2.00 sf

Primary OutFlow Max=0.07 cfs @ 12.53 hrs HW=159.05' TW=158.10' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.07 cfs @ 0.70 fps)

Summary for Link 1L: OVERFLOW

Inflow Are	a =	782,870 sf,	7.42% Impervious,	Inflow Depth = 0.00"	for NRCC 2YR 24H event
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Summary for Link 10L: OFF-SITE

Inflow Are	a =	258,650 sf,	0.00% Impervious,	Inflow Depth = 0.00" for NRCC 2YR 24H event
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment1S: AREA OUTSIDE	Runoff Area=258,650 sf 0.00% Impervious Runoff Depth=0.62" Tc=5.0 min CN=32 Runoff=1.49 cfs 13,345 cf
Subcatchment 2S: AREA OUTSIDE	Runoff Area=285,632 sf 0.00% Impervious Runoff Depth=0.54" w Length=491' Tc=12.3 min CN=31 Runoff=1.19 cfs 12,860 cf
Subcatchment 3S: S SWALE 1	Runoff Area=21,901 sf 0.00% Impervious Runoff Depth=3.03"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=56 Runoff=1.72 cfs 5,536 cf
Subcatchment 4S: S SWALE 2	Runoff Area=37,407 sf 0.00% Impervious Runoff Depth=1.44" Tc=5.0 min CN=41 Runoff=1.13 cfs 4,479 cf
Subcatchment 5S: S SWALE 3	Runoff Area=28,531 sf 0.00% Impervious Runoff Depth=2.16"
Flow Length=644'	Slope=0.0200 '/' Tc=9.3 min CN=48 Runoff=1.32 cfs 5,131 cf
Subcatchment6S: ACCESS RAMP	Runoff Area=17,331 sf 0.00% Impervious Runoff Depth=5.58" Tc=5.0 min CN=78 Runoff=2.67 cfs 8,053 cf
Subcatchment7S: S SWALE 4	Runoff Area=20,291 sf 0.00% Impervious Runoff Depth=3.83"
Flow Length=644'	Slope=0.0200 '/' Tc=9.3 min CN=63 Runoff=1.85 cfs 6,470 cf
Subcatchment 8S: N SWALE 1	Runoff Area=15,778 sf 0.00% Impervious Runoff Depth=5.81"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=80 Runoff=2.41 cfs 7,643 cf
Subcatchment9S: N SWALE 2	Runoff Area=14,101 sf 0.00% Impervious Runoff Depth=6.17"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=83 Runoff=2.26 cfs 7,249 cf
Subcatchment 10S: N SWALE 3	Runoff Area=12,739 sf 0.00% Impervious Runoff Depth=6.17"
Flow Length=378'	Slope=0.0200 '/' Tc=6.2 min CN=83 Runoff=2.04 cfs 6,548 cf
Subcatchment11S: NW PERF. DRAIN	Runoff Area=45,853 sf 0.00% Impervious Runoff Depth=3.83"
Flow Length=660'	Slope=0.0200 '/' Tc=6.0 min CN=63 Runoff=4.70 cfs 14,622 cf
Subcatchment 12S: CENTRAL PERF. DRA	■ Runoff Area=6,658 sf 0.00% Impervious Runoff Depth=3.83 Tc=5.0 min CN=63 Runoff=0.71 cfs 2,123 cf
Subcatchment13S: GIS BUILDINGS	Runoff Area=21,900 sf 100.00% Impervious Runoff Depth=7.96" Tc=5.0 min CN=98 Runoff=4.19 cfs 14,527 cf
Subcatchment 14S: GIS BLDGS PERF	Runoff Area=74,801 sf 0.00% Impervious Runoff Depth=3.83"
Flow Length=570'	Slope=0.0200 '/' Tc=8.4 min CN=63 Runoff=7.05 cfs 23,852 cf
	N Runoff Area=54,087 sf 0.00% Impervious Runoff Depth=3.83" Slope=0.0200 '/' Tc=8.4 min CN=63 Runoff=5.10 cfs 17,247 cf
Subcatchment 16S: N PERF. DRAIN	Runoff Area=35,583 sf 0.57% Impervious Runoff Depth=3.83" Tc=5.0 min CN=63 Runoff=3.78 cfs 11,347 cf

CWW Substation 5-Parcel Proposed Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20" Prepared by Stantec Consulting Ltd. Printed 2/10/2023 HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLC Page 114 Runoff Area=41,546 sf 20.00% Impervious Runoff Depth=4.64" Subcatchment 17S: SE PERF. DRAIN 1 Slope=0.0200 '/' Tc=5.9 min CN=70 Runoff=5.21 cfs 16,053 cf Flow Length=425' Subcatchment 18S: SE PERF, DRAIN 2 Runoff Area=25,304 sf 16.82% Impervious Runoff Depth=4.52" Tc=5.0 min CN=69 Runoff=3.19 cfs 9,531 cf Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=7.96" Subcatchment 20S: SR4 Tc=5.0 min CN=98 Runoff=0.33 cfs 1,158 cf Subcatchment21S: XFMR 1 Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=7.96" Tc=5.0 min CN=98 Runoff=0.52 cfs 1,787 cf Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=7.96" Subcatchment 22S: SR5 Tc=5.0 min CN=98 Runoff=0.33 cfs 1,158 cf Subcatchment 23S: XFMR 2 Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=7.96" Tc=5.0 min CN=98 Runoff=0.52 cfs 1,787 cf Runoff Area=2,694 sf 100.00% Impervious Runoff Depth=7.96" Subcatchment 24S: XFMR 3 Tc=5.0 min CN=98 Runoff=0.52 cfs 1,787 cf Subcatchment 25S: SR6 Runoff Area=1,745 sf 100.00% Impervious Runoff Depth=7.96" Tc=5.0 min CN=98 Runoff=0.33 cfs 1,158 cf Subcatchment 26S: SR1 Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=7.96" Tc=5.0 min CN=98 Runoff=0.31 cfs 1,061 cf Subcatchment 27S: SR2 Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=7.96" Tc=5.0 min CN=98 Runoff=0.31 cfs 1,061 cf Subcatchment 28S: SR3 Runoff Area=1,600 sf 100.00% Impervious Runoff Depth=7.96" Tc=5.0 min CN=98 Runoff=0.31 cfs 1,061 cf Subcatchment 29S: STATCOM 1 Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=7.96" Tc=5.0 min CN=98 Runoff=0.34 cfs 1,174 cf Subcatchment 30S: STATCOM 2 Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=7.96" Tc=5.0 min CN=98 Runoff=0.34 cfs 1,174 cf Subcatchment 31S: STATCOM 3 Runoff Area=1,770 sf 100.00% Impervious Runoff Depth=7.96" Tc=5.0 min CN=98 Runoff=0.34 cfs 1,174 cf Reach 1R: S SWALE PART 1 Avg. Flow Depth=0.50' Max Vel=2.21 fps Inflow=1.72 cfs 5,536 cf n=0.030 L=309.0' S=0.0137 '/' Capacity=10.56 cfs Outflow=1.63 cfs 5,536 cf Reach 2R: S SWALE PART 2 Avg. Flow Depth=0.56' Max Vel=2.85 fps Inflow=2.72 cfs 10,015 cf n=0.030 L=257.3' S=0.0195 '/' Capacity=12.62 cfs Outflow=2.65 cfs 10.015 cf Avg. Flow Depth=0.68' Max Vel=2.77 fps Inflow=3.97 cfs 15,146 cf **Reach 3R: S SWALE PART 3** n=0.030 L=329.0' S=0.0142 '/' Capacity=10.77 cfs Outflow=3.83 cfs 15,146 cf **Reach 4R: S SWALE PART 4** Avg. Flow Depth=0.98' Max Vel=2.54 fps Inflow=7.57 cfs 29,670 cf

n=0.030 L=367.8' S=0.0073 '/' Capacity=9.98 cfs Outflow=7.30 cfs 29.670 cf

CWW Substation 5-Parcel Proposed Type III 24-hr RMAT 50-YR 24H	H TIER 3 Rainfall=8.20"
Prepared by Stantec Consulting Ltd.	Printed 2/10/2023
HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLC	Page 115

Reach 5R: DRAINAGE CHANNEL Avg. Flow Depth=0.40' Max Vel=5.61 fps Inflow=45.25 cfs 159,166 cf n=0.078 L=175.0' S=0.3086 '/' Capacity=198.63 cfs Outflow=45.12 cfs 159,166 cf
Reach 11R: N SWALE PART 1 Avg. Flow Depth=0.55' Max Vel=2.49 fps Inflow=2.41 cfs 7,643 cf n=0.030 L=447.0' S=0.0153 '/' Capacity=11.19 cfs Outflow=2.23 cfs 7,643 cf
Reach 12R: N SWALE PART 2 Avg. Flow Depth=0.70' Max Vel=2.80 fps Inflow=4.40 cfs 14,891 cf n=0.030 L=431.0' S=0.0137 '/' Capacity=10.59 cfs Outflow=4.17 cfs 14,891 cf
Reach 13R: N SWALE PART 3 Avg. Flow Depth=0.81' Max Vel=2.98 fps Inflow=6.03 cfs 21,440 cf n=0.030 L=386.0' S=0.0131 '/' Capacity=10.33 cfs Outflow=5.82 cfs 21,440 cf
Pond 1P: DETENTION AREA - Peak Elev=90.51' Storage=106,144 cf Inflow=45.16 cfs 172,026 cf Discarded=2.11 cfs 172,026 cf Primary=0.00 cfs 0 cf Outflow=2.11 cfs 172,026 cf
Pond 2P: SEDIMENT FOREBAY Peak Elev=150.39' Storage=9,783 cf Inflow=13.08 cfs 51,110 cf Discarded=0.14 cfs 16,787 cf Primary=12.80 cfs 34,324 cf Outflow=12.94 cfs 51,111 cf
Pond 3P: DMHs w/ Vortex Unit Peak Elev=150.69' Inflow=37.29 cfs 124,842 cf 36.0" Round Culvert n=0.011 L=25.0' S=0.1200 '/' Outflow=37.29 cfs 124,842 cf
Pond 4P: DMH Peak Elev=152.99' Inflow=37.29 cfs 124,842 cf 36.0" Round Culvert n=0.011 L=36.0' S=0.0639 '/' Outflow=37.29 cfs 124,842 cf
Pond 5P: DMH Peak Elev=158.79' Inflow=23.40 cfs 81,205 cf 24.0" Round Culvert n=0.011 L=263.0' S=0.0186 '/' Outflow=23.40 cfs 81,205 cf
Pond 6P: OIL/WATER SEPARATOR Peak Elev=158.82' Inflow=2.55 cfs 8,834 cf 24.0" Round Culvert n=0.012 L=27.0' S=0.0148 '/' Outflow=2.55 cfs 8,834 cf
Pond 7P: DMH Peak Elev=159.15' Inflow=2.55 cfs 8,834 cf 12.0" Round Culvert n=0.011 L=40.0' S=0.0100 '/' Outflow=2.55 cfs 8,834 cf
Pond 8P: DMH Peak Elev=159.41' Inflow=1.70 cfs 5,889 cf 12.0" Round Culvert n=0.011 L=154.0' S=0.0065 '/' Outflow=1.70 cfs 5,889 cf
Pond 9P: DMH Peak Elev=159.85' Inflow=0.85 cfs 2,945 cf 12.0" Round Culvert n=0.011 L=141.0' S=0.0092 '/' Outflow=0.85 cfs 2,945 cf
Pond 10P: 24" Petro-Barrier Peak Elev=162.81' Inflow=0.33 cfs 1,158 cf 6.0" Round Culvert n=0.010 L=30.0' S=0.0143 '/' Outflow=0.33 cfs 1,158 cf
Pond 11P: 24" Petro-Barrier Peak Elev=160.17' Inflow=0.52 cfs 1,787 cf 6.0" Round Culvert n=0.010 L=18.0' S=0.0122 '/' Outflow=0.52 cfs 1,787 cf
Pond 12P: 24" Petro-Barrier Peak Elev=161.08' Inflow=0.33 cfs 1,158 cf 6.0" Round Culvert n=0.010 L=32.0' S=0.0094 '/' Outflow=0.33 cfs 1,158 cf
Pond 13P: 24" Petro-Barrier Peak Elev=159.60' Inflow=0.52 cfs 1,787 cf 6.0" Round Culvert n=0.010 L=20.0' S=0.0115 '/' Outflow=0.52 cfs 1,787 cf

CWW Substation 5-Parcel Proposed Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20" Prepared by Stantec Consulting Ltd. Printed 2/10/2023 HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLC Page 116 Peak Elev=159.36' Inflow=0.52 cfs 1,787 cf Pond 14P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=21.0' S=0.0133 '/' Outflow=0.52 cfs 1,787 cf Peak Elev=159.25' Inflow=0.33 cfs 1,158 cf Pond 15P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=34.0' S=0.0076 '/' Outflow=0.33 cfs 1,158 cf Peak Elev=161.54' Inflow=5.40 cfs 16,745 cf Pond 16P: DMH 12.0" Round Culvert n=0.011 L=17.0' S=0.0294 '/' Outflow=5.40 cfs 16,745 cf Peak Elev=161.48' Inflow=15.74 cfs 55,627 cf Pond 17P: DMH 24.0" Round Culvert n=0.011 L=88.2' S=0.0442 '/' Outflow=15.74 cfs 55,627 cf Peak Elev=164.29' Inflow=12.14 cfs 41,100 cf Pond 18P: DMH 18.0" Round Culvert n=0.011 L=92.0' S=0.0217 '/' Outflow=12.14 cfs 41,100 cf Pond 20P: DMH Peak Elev=153.81' Inflow=12.15 cfs 36,931 cf 24.0" Round Culvert n=0.011 L=56.0' S=0.0286 '/' Outflow=12.15 cfs 36,931 cf Peak Elev=156.15' Inflow=3.19 cfs 9,531 cf Pond 21P: DMH 12.0" Round Culvert n=0.011 L=62.0' S=0.0410 '/' Outflow=3.19 cfs 9,531 cf Peak Elev=157.36' Inflow=8.97 cfs 27,399 cf Pond 22P: DMH 18.0" Round Culvert n=0.011 L=56.0' S=0.0089 '/' Outflow=8.97 cfs 27,399 cf Peak Elev=153.49' Inflow=1.94 cfs 6,706 cf Pond 23P: OIL/WATER SEPARATOR 12.0" Round Culvert n=0.011 L=182.0' S=0.0093 '/' Outflow=1.98 cfs 6,706 cf Peak Elev=153.71' Inflow=1.94 cfs 6,706 cf Pond 24P: DMH 12.0" Round Culvert n=0.011 L=1.0' S=0.1000 '/' Outflow=1.94 cfs 6,706 cf Peak Elev=153.87' Inflow=0.34 cfs 1,174 cf Pond 25P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=58.0' S=0.0102 '/' Outflow=0.34 cfs 1,174 cf Pond 26P: 24" Petro-Barrier Peak Elev=153.82' Inflow=0.34 cfs 1,174 cf 6.0" Round Culvert n=0.010 L=17.0' S=0.0576 '/' Outflow=0.34 cfs 1,174 cf Peak Elev=154.58' Inflow=1.26 cfs 4,358 cf Pond 27P: DMH 12.0" Round Culvert n=0.011 L=224.0' S=0.0085 '/' Outflow=1.26 cfs 4,358 cf Pond 28P: DMH Peak Elev=155.03' Inflow=1.26 cfs 4,358 cf 12.0" Round Culvert n=0.011 L=50.0' S=0.0080 '/' Outflow=1.26 cfs 4,358 cf Peak Elev=155.33' Inflow=0.34 cfs 1,174 cf Pond 29P: 24" Petro-Barrier 6.0" Round Culvert n=0.010 L=7.0' S=0.0643 '/' Outflow=0.34 cfs 1,174 cf Peak Elev=160.89' Inflow=0.92 cfs 3,184 cf Pond 30P: DMH 12.0" Round Culvert n=0.011 L=98.0' S=0.0092 '/' Outflow=0.92 cfs 3,184 cf Peak Elev=162.40' Inflow=0.92 cfs 3,184 cf Pond 31P: DMH 12.0" Round Culvert n=0.011 L=232.0' S=0.0060 '/' Outflow=0.92 cfs 3,184 cf Pond 32P: 24" Petro-Barrier Peak Elev=162.80' Inflow=0.31 cfs 1,061 cf 6.0" Round Culvert n=0.010 L=10.0' S=0.0240 '/' Outflow=0.31 cfs 1,061 cf

Pond 33P: DMH	Peak Elev=163.59' Inflow=0.61 cfs 2,123 cf 12.0" Round Culvert n=0.011 L=159.0' S=0.0075 '/' Outflow=0.61 cfs 2,123 cf
Pond 34P: 24" Petro-Barr	ier Peak Elev=163.80' Inflow=0.31 cfs 1,061 cf 6.0" Round Culvert n=0.010 L=8.0' S=0.0162 '/' Outflow=0.31 cfs 1,061 cf
Pond 35P: 24" Petro-Barr	ier Peak Elev=164.75' Inflow=0.31 cfs 1,061 cf 6.0" Round Culvert n=0.010 L=38.0' S=0.0103 '/' Outflow=0.31 cfs 1,061 cf
Pond 40P: DMH	Peak Elev=159.08' Inflow=5.72 cfs 23,199 cf 24.0" x 12.0" Box Culvert n=0.011 L=10.0' S=0.0300 '/' Outflow=5.72 cfs 23,199 cf
Pond 41P: CULVERT	Peak Elev=159.71' Inflow=3.83 cfs 15,146 cf 24.0" x 12.0" Box Culvert n=0.011 L=55.0' S=0.0182 '/' Outflow=3.83 cfs 15,146 cf
Link 1L: OVERFLOW	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Link 10L: OFF-SITE	Inflow=1.49 cfs 13,345 cf Primary=1.49 cfs 13,345 cf
Total Dupoff Ara	a = 1.041.520 of Bunoff Volume = 202.157 of Average Bunoff Depth = 2.22

Total Runoff Area = 1,041,520 sf Runoff Volume = 202,157 cfAverage Runoff Depth = 2.33"94.42% Pervious = 983,426 sf5.58% Impervious = 58,094 sf

Summary for Subcatchment 1S: AREA OUTSIDE SUBSTATION - NOT TO POND

Runoff = 1.49 cfs @ 12.32 hrs, Volume= 13,345 cf, Depth= 0.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

A	rea (sf)	CN	Description		
1	79,269	30	Woods, Good, HSG A		
	72,994	30	Meadow, non-grazed, HSG A		
	6,387	96	Gravel surfa	ace, HSG A	λ
2	58,650	32	32 Weighted Average		
2	258,650 100.00% I		100.00% Pe	ervious Are	a
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)	
5.0					Direct Entry, Direct Entry

Summary for Subcatchment 2S: AREA OUTSIDE SUBSTATION - TO POND

Runoff	=	1.19 cfs @	12.47 hrs, Volume=	12,860 cf, Depth= 0.54"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

A	rea (sf)	CN I	Description		
2	206,991	30 \	Noods, Go	od, HSG A	
	72,294	30 I	Meadow, no	on-grazed,	HSG A
	6,347	96 (Gravel surfa	ace, HSG A	Ι
2	285,632	31 \	Neighted A	verage	
2	285,632		100.00% Pe	ervious Are	а
Тс	Length	Slope	,	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.4	50	0.5000	0.25		Sheet Flow, Sheet
					Woods: Light underbrush n= 0.400 P2= 3.29"
8.9	441	0.1100	0.83		Shallow Concentrated Flow, Shallow Conc
					Forest w/Heavy Litter Kv= 2.5 fps
12.3	491	Total			

Summary for Subcatchment 3S: S SWALE 1

Runoff = 1.72 cfs @ 12.10 hrs, Volume= 5,536 cf, Depth= 3.03"

A	rea (sf)	CN D	escription							
*	3,599	63 C	63 Crushed Stone Surface, HSG A							
	6,746	96 G	Gravel surfa	ace, HSG A	N Contraction of the second seco					
	7,202	30 V	Voods, Go	od, HSG A						
	4,354	30 N	leadow, no	on-grazed,	HSG A					
	21,901	56 V	Veighted A	verage						
	21,901		0	ervious Are	а					
	,									
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
2.3	50	0.0200	0.36	· · ·	Sheet Flow, Sheet					
					Fallow n= 0.050 P2= 3.29"					
3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc					
					Nearly Bare & Untilled Kv= 10.0 fps					
6.2	378	Total								
0.2	010	1 O toli								
			Summar		catchment 4S: S SWALE 2					
		•	Summar	y 101 300	catchinent 45. 5 SWALE 2					
Runoff	=	1.13 cf	s@ 12.1	0 hrs, Volu	ime= 4,479 cf, Depth= 1.44"					
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"									

	A	rea (sf)	CN	Description						
		5,160	96	Gravel surfa	Gravel surface, HSG A					
*		2,578	63	Crushed St	one Surfac	e, HSG A				
		20,025	30	Woods, Go	od, HSG A					
		9,644	30	Meadow, no	on-grazed,	HSG A				
		37,407	41	Weighted A	verage					
	:	37,407		100.00% Pe	ervious Are	а				
	Тс	Length	Slop	,	Capacity	Description				
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)					
	5.0					Direct Entry, Direct Entry				

Summary for Subcatchment 5S: S SWALE 3

Runoff = 1.32 cfs @ 12.14 hrs, Volume= 5,131 cf, Depth= 2.16"

	Area (sf)	CN	Description
*	3,335	63	Crushed Stone Surface, HSG A
	7,061	30	Woods, Good, HSG A
	6,315	96	Gravel surface, HSG A
	11,820	30	Meadow, non-grazed, HSG A
	28,531	48	Weighted Average
	28,531		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.3	50	0.0200	0.36		Sheet Flow, Sheet
					Fallow n= 0.050 P2= 3.29"
7.0	594	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc
					Nearly Bare & Untilled Kv= 10.0 fps
03	644	Total			

9.3 644 Total

Summary for Subcatchment 6S: ACCESS RAMP

Runoff 2.67 cfs @ 12.07 hrs, Volume= 8,053 cf, Depth= 5.58" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

Area (sf)	CN	Description		
12,617	96	Gravel surfa	ace, HSG A	N
1,094	30	Woods, Go	od, HSG A	
3,620	30	Meadow, no	on-grazed,	HSG A
17,331	78	Weighted A	verage	
17,331		100.00% Pe	ervious Are	а
Tc Length (min) (feet)	Slop (ft/		Capacity (cfs)	Description
5.0				Direct Entry, Direct Entry

Summary for Subcatchment 7S: S SWALE 4

1.85 cfs @ 12.13 hrs, Volume= 6,470 cf, Depth= 3.83" Runoff =

_	А	rea (sf)	CN [Description							
*		6,230	63 (63 Crushed Stone Surface, HSG A							
		7,051	96 (Gravel surfa	ace, HSG A	N Contraction of the second seco					
_		7,010	30 I	Aeadow, no	on-grazed,	HSG A					
		20,291	63 \	3 Weighted Average							
		20,291		100.00% Pe	ervious Are	а					
	Tc	Length	Slope		Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	2.3	50	0.0200	0.36		Sheet Flow, Sheet					
						Fallow n= 0.050 P2= 3.29"					
	7.0	594	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc					
						Nearly Bare & Untilled Kv= 10.0 fps					
	9.3	644	Total								

Summary for Subcatchment 8S: N SWALE 1

Runoff = 2.41 cfs @ 12.09 hrs, Volume= 7,643 cf, Depth= 5.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

	A	rea (sf)	CN [Description						
*		4,949	63 (63 Crushed Stone Surface, HSG A						
		9,543	96 (Gravel surfa	ace, HSG A	A Í				
		1,286	30 I	Meadow, non-grazed, HSG A						
		15,778	8 80 Weighted Average							
		15,778		100.00% Pe	ervious Are	a				
	Тс	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	2.3	50	0.0200	0.36		Sheet Flow, Sheet				
						Fallow n= 0.050 P2= 3.29"				
	3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc				
_						Nearly Bare & Untilled Kv= 10.0 fps				
	<u> </u>	070	Tatal							

6.2 378 Total

Summary for Subcatchment 9S: N SWALE 2

Runoff = 2.26 cfs @ 12.09 hrs, Volume= 7,249 cf, Depth= 6.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

_	A	rea (sf)	CN I	Description					
*		5,475	63	Crushed Stone Surface, HSG A					
		8,626	96	Gravel surfa	ace, HSG A	Α			
14,101 83 Weighted Average									
14,101 100.00% Pervious Area					ervious Are	а			
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
	2.3	50	0.0200	0.36		Sheet Flow, Sheet			
	3.9	328	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps			

6.2 378 Total

Summary for Subcatchment 10S: N SWALE 3

Runoff = 2.04 cfs @ 12.09 hrs, Volume= 6,548 cf, Depth= 6.17"

_	A	rea (sf)	CN [Description						
*		4,958	63 (Crushed Stone Surface, HSG A						
_		7,781	96 (Gravel surfa	ace, HSG A	Α				
12,739 83 Weighted Average										
		12,739		00.00% Pe	ervious Are	а				
	Tc	Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	2.3	50	0.0200	0.36		Sheet Flow, Sheet				
						Fallow n= 0.050 P2= 3.29"				
	3.9	328	0.0200	1.41		Shallow Concentrated Flow, Shallow Conc				
_						Nearly Bare & Untilled Kv= 10.0 fps				
	6.2	378	Total							

Summary for Subcatchment 11S: NW PERF. DRAIN

Runoff = 4.70 cfs @ 12.09 hrs, Volume= 14,622 cf, Depth= 3.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

_	A	rea (sf)	CN E	Description		
*		45,853	63 C	Crushed St	one Surface	e, HSG A
		45,853	1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.3	50	0.0200	0.36		Sheet Flow, Sheet
	3.0	255	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps
	0.7	355	0.0200	8.34	6.55	Pipe Channel, Perf Pipe
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
_	6.0	660	Total			

Summary for Subcatchment 12S: CENTRAL PERF. DRAIN 2

Runoff = 0.71 cfs @ 12.08 hrs, Volume= 2,123 cf, Depth= 3.83"

	Area (sf)	CN	Description
*	6,658	63	Crushed Stone Surface, HSG A
	6,658		100.00% Pervious Area

CWW Substation 5-Parcel Proposed Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"Prepared by Stantec Consulting Ltd.Printed 2/10/2023HydroCAD® 10.00-25 s/n 01807 © 2019 HydroCAD Software Solutions LLCPage 123									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
5.0 Direct Entry, Direct Entry									
Summary for Subcatchment 13S: GIS BUILDINGS									
Runoff = 4.19 cfs @ 12.07 hrs, Volume= 14,527 cf, Depth= 7.96"									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"									
Area (sf) CN Description									
21,900 98 Roofs, HSG A									
21,900 100.00% Impervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
5.0 Direct Entry, Direct Entry									
Summary for Subcatchment 14S: GIS BLDGS PERF DRAIN									
Runoff = 7.05 cfs @ 12.12 hrs, Volume= 23,852 cf, Depth= 3.83"									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"									
Area (sf) CN Description									
* 74,801 63 Crushed Stone Surface, HSG A									
74,801 100.00% Pervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
2.3 50 0.0200 0.36 Sheet Flow, Sheet									
6.1 520 0.0200 1.41 Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps									
8.4 570 Total									
Summary for Subcatchment 15S: CENTRAL PERF. DRAIN 1									

Runoff = 5.10 cfs @ 12.12 hrs, Volume= 17,247 cf, Depth= 3.83"

	Area (sf)	CN	Description
*	54,087	63	Crushed Stone Surface, HSG A
	54,087		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.3	50	0.0200	0.36		Sheet Flow, Sheet
6.1	520	0.0200	1.41		Fallow n= 0.050 P2= 3.29" Shallow Concentrated Flow, Shallow Conc Nearly Bare & Untilled Kv= 10.0 fps
8.4	570	Total			

Summary for Subcatchment 16S: N PERF. DRAIN

Runoff = 3.78 cfs @ 12.08 hrs, Volume= 11,347 cf, Depth= 3.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

_	А	rea (sf)	CN	Description		
*		35,381	63	Crushed St		e, HSG A
_		202	98	Roofs, HSC	β A	
		35,583	63	Weighted A	verage	
		35,381		99.43% Per		
		202		0.57% Impe	ervious Area	а
	Tc	Length	Slop	e Velocity	Capacity	Description
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	5.0					Direct Entry, Direct Entry

Summary for Subcatchment 17S: SE PERF. DRAIN 1

Runoff = 5.21 cfs @ 12.09 hrs, Volume= 16,053 cf, Depth= 4.64"

	A	rea (sf)	CN [Description							
		8,310	98 F	98 Roofs, HSG A							
*		33,236	63 (Crushed St	one Surface	e, HSG A					
		41,546	70 \	Neighted A	verage						
		33,236	8	30.00% Pei	vious Area						
		8,310	2	20.00% Imp	pervious Are	ea					
	Тс	Length	Slope		Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	2.3	50	0.0200	0.36		Sheet Flow, Sheet					
						Fallow n= 0.050 P2= 3.29"					
	3.5 300 0.0200 1.41 Sha			Shallow Concentrated Flow, Shallow							
						Nearly Bare & Untilled Kv= 10.0 fps					
	0.1	75	0.0200	8.34	6.55	Pipe Channel, Perf. Pipe					
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'					
						n= 0.010 PVC, smooth interior					
	5.9	425	Total								

Summary for Subcatchment 18S: SE PERF. DRAIN 2

Runoff = 3.19 cfs @ 12.07 hrs, Volume= 9,531 cf, Depth= 4.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

	A	rea (sf)	CN	Description		
		4,255	98	Roofs, HSG	βA	
*		21,049	63	Crushed St	one Surface	e, HSG A
		25,304 21,049 4,255	69	Weighted A 83.18% Per 16.82% Imp	vious Area	
	Tc	Length	Slope	,	Capacity	Description
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	5.0					Direct Entry, Direct Entry

Summary for Subcatchment 20S: SR4

Runoff = 0.33 cfs @ 12.07 hrs, Volume= 1,158 cf, Depth= 7.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

Α	rea (sf)	CN [Description							
	1,745	98 (98 Concrete Containment							
	1,745		100.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
5.0					Direct Entry, Direct Entry					

Summary for Subcatchment 21S: XFMR 1

Runoff = 0.52 cfs @ 12.07 hrs, Volume= 1,787 cf, Depth= 7.96"

A	rea (sf)	CN [I Description						
	2,694	98 (Concrete Containment						
	2,694	1	00.00% Im	pervious A	rea				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry, Direct Entry				

Summary for Subcatchment 22S: SR5

Runoff = 0.33 cfs @ 12.07 hrs, Volume= 1,158 cf, Depth= 7.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

А	rea (sf)	CN [Description				
	1,745	98 (Concrete C	ontainment	t		
	1,745		100.00% In	npervious A	rea		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry	/, Direct E	Entry
			Summa	ry for Su	bcatchmen	nt 23S: X	FMR 2
Runoff	=	0.52 c	fs @ 12.0	7 hrs, Volu	ime=	1,787 cf,	Depth= 7.96"
				CS, Weigh R 3 Rainfal		Span= 0.	00-72.00 hrs, dt= 0.01 hrs
A	rea (sf)	CN [Description				
	2,694	98 (Concrete C	ontainment	t		
	2,694		100.00% In	npervious A	vrea		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry	/, Direct E	Entry
			Summa	ry for Su	bcatchmen	nt 24S: X	FMR 3
Runoff	=	0.52 c	fs @ 12.0	7 hrs, Volu	ıme=	1,787 cf,	Depth= 7.96"
				CS, Weigh R 3 Rainfal		Span= 0.0	00-72.00 hrs, dt= 0.01 hrs
A	rea (sf)	CN [Description				
	2,694	98 (Concrete C	ontainment	t		
	2,694		100.00% Im	npervious A	rea		

Description

Direct Entry, Direct Entry

Slope Velocity

(ft/sec)

(ft/ft)

Тс

(min)

5.0

Length

(feet)

Capacity

(cfs)

Summary for Subcatchment 25S: SR6

Runoff = 0.33 cfs @ 12.07 hrs, Volume= 1,158 cf, Depth= 7.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

Area (sf) CN Description
1,745 98 Concrete Containment
1,745 100.00% Impervious Area
Tc Length Slope Velocity Capacity Description
(min) (feet) (ft/ft) (ft/sec) (cfs)
5.0 Direct Entry, Direct Entry
Summary for Subcatchment 26S: SR1
Runoff = 0.31 cfs @ 12.07 hrs, Volume= 1,061 cf, Depth= 7.96"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"
Area (sf) CN Description
1,600 98 Concrete Containment
1,600 100.00% Impervious Area
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
5.0 Direct Entry, Direct Entry
Summary for Subcatchment 27S: SR2
Runoff = 0.31 cfs @ 12.07 hrs, Volume= 1,061 cf, Depth= 7.96"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"
Area (sf) CN Description
1,600 98 Concrete Containment
1,600 100.00% Impervious Area
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)

Direct Entry, Direct Entry

5.0

Summary for Subcatchment 28S: SR3

Runoff = 0.31 cfs @ 12.07 hrs, Volume= 1,061 cf, Depth= 7.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"

Area (sf) CN Description
1,600 98 Concrete Containment
1,600 100.00% Impervious Area
Tc Length Slope Velocity Capacity Description
(min) (feet) (ft/sec) (cfs) 5.0 Direct Entry, Direct Entry
5.0 Direct Entry, Direct Entry
Summary for Subcatchment 29S: STATCOM 1 TRANSFORMER
Runoff = 0.34 cfs @ 12.07 hrs, Volume= 1,174 cf, Depth= 7.96"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"
Area (sf) CN Description
1,770 98 Concrete Containment
1,770 100.00% Impervious Area
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
5.0 Direct Entry, Direct Entry
Summary for Subcatchment 30S: STATCOM 2 TRANSFORMER
Runoff = 0.34 cfs @ 12.07 hrs, Volume= 1,174 cf, Depth= 7.96"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"
Area (sf) CN Description
1,770 98 Concrete Containment
1,770 100.00% Impervious Area
Tc Length Slope Velocity Capacity Description

Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)

(feet)

Direct Entry, Direct Entry

Summary for Subcatchment 31S: STATCOM 3 TRANSFORMER

Runoff = 0.34 cfs @ 12.07 hrs, Volume= 1,174 cf, Depth= 7.96"

Area (sf) CN Description
1,770 98 Concrete Containment
1,770 100.00% Impervious Area
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
5.0 Direct Entry, Direct Entry
Summary for Reach 1R: S SWALE PART 1
Inflow Area = 21,901 sf, 0.00% Impervious, Inflow Depth = 3.03" for RMAT 50-YR 24H TIER 3 event Inflow = 1.72 cfs @ 12.10 hrs, Volume= 5,536 cf Outflow = 1.63 cfs @ 12.13 hrs, Volume= 5,536 cf, Atten= 5%, Lag= 1.7 min
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.21 fps, Min. Travel Time= 2.3 min Avg. Velocity = 0.91 fps, Avg. Travel Time= 5.7 min
Peak Storage= 228 cf @ 12.13 hrs Average Depth at Peak Storage= 0.50' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 38.49 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.56 cfs
0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 309.0' Slope= 0.0137 '/' Inlet Invert= 174.13', Outlet Invert= 169.91'

Summary for Reach 2R: S SWALE PART 2

Inflow Area	=	59,308 sf, 0.00% Impervious, Inflow Depth = 2.03" for RMAT 50-YR 24H TIER 3 event
Inflow	=	2.72 cfs @ 12.11 hrs, Volume= 10,015 cf
Outflow	=	2.65 cfs @ 12.13 hrs, Volume= 10,015 cf, Atten= 2%, Lag= 1.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.85 fps, Min. Travel Time= 1.5 min Avg. Velocity = 1.19 fps, Avg. Travel Time= 3.6 min

Peak Storage= 239 cf @ 12.13 hrs Average Depth at Peak Storage= 0.56' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 46.01 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 12.62 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 257.3' Slope= 0.0195 '/' Inlet Invert= 169.90', Outlet Invert= 164.88'

Summary for Reach 3R: S SWALE PART 3

 Inflow Area =
 87,839 sf, 0.00% Impervious, Inflow Depth = 2.07" for RMAT 50-YR 24H TIER 3 event

 Inflow =
 3.97 cfs @ 12.14 hrs, Volume=
 15,146 cf

 Outflow =
 3.83 cfs @ 12.16 hrs, Volume=
 15,146 cf, Atten= 3%, Lag= 1.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.77 fps, Min. Travel Time= 2.0 min Avg. Velocity = 1.12 fps, Avg. Travel Time= 4.9 min

Peak Storage= 455 cf @ 12.16 hrs Average Depth at Peak Storage= 0.68' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 39.24 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.77 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 329.0' Slope= 0.0142 '/' Inlet Invert= 164.80', Outlet Invert= 160.13'

Summary for Reach 4R: S SWALE PART 4

Inflow Area = 125,461 sf, 0.00% Impervious, Inflow Depth = 2.84" for RMAT 50-YR 24H TIER 3 event Inflow = 7.57 cfs @ 12.13 hrs, Volume= 29,670 cf Outflow = 7.30 cfs @ 12.16 hrs, Volume= 29,670 cf, Atten= 4%, Lag= 1.9 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Max. Velocity= 2.54 fps, Min. Travel Time= 2.4 min Avg. Velocity = 0.90 fps, Avg. Travel Time= 6.8 min

Peak Storage= 1,055 cf @ 12.16 hrs Average Depth at Peak Storage= 0.98' Defined Flood Depth= 2.00' Flow Area= 9.5 sf, Capacity= 31.59 cfs Bank-Full Depth= 1.10' Flow Area= 3.6 sf, Capacity= 9.98 cfs

0.00' x 1.10' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.60' Length= 367.8' Slope= 0.0073 '/' Inlet Invert= 157.70', Outlet Invert= 155.00'

Summary for Reach 5R: DRAINAGE CHANNEL

 Inflow Area =
 497,238 sf, 11.68% Impervious, Inflow Depth = 3.84" for RMAT 50-YR 24H TIER 3 event

 Inflow =
 45.25 cfs @ 12.13 hrs, Volume=
 159,166 cf

 Outflow =
 45.12 cfs @ 12.13 hrs, Volume=
 159,166 cf, Atten= 0%, Lag= 0.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 5.61 fps, Min. Travel Time= 0.5 min Avg. Velocity = 1.19 fps, Avg. Travel Time= 2.5 min

Peak Storage= 1,407 cf @ 12.13 hrs Average Depth at Peak Storage= 0.40' Defined Flood Depth= 1.00' Flow Area= 20.0 sf, Capacity= 198.63 cfs Bank-Full Depth= 1.00' Flow Area= 20.0 sf, Capacity= 198.63 cfs

20.00' x 1.00' deep channel, n= 0.078 Riprap, 12-inch Length= 175.0' Slope= 0.3086 '/' Inlet Invert= 145.00', Outlet Invert= 91.00'

Summary for Reach 11R: N SWALE PART 1

Inflow Area = 15.778 sf. 0.00% Impervious. Inflow Depth = 5.81" for RMAT 50-YR 24H TIER 3 event Inflow 2.41 cfs @ 12.09 hrs. Volume= 7.643 cf = 2.23 cfs @ 12.12 hrs, Volume= Outflow = 7,643 cf, Atten= 8%, Lag= 2.0 min Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.49 fps, Min. Travel Time= 3.0 min Avg. Velocity = 0.92 fps, Avg. Travel Time= 8.1 min Peak Storage= 400 cf @ 12.12 hrs Average Depth at Peak Storage= 0.55' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 40.78 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 11.19 cfs 0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 447.0' Slope= 0.0153 '/' Inlet Invert= 174.03', Outlet Invert= 167.18'

Summary for Reach 12R: N SWALE PART 2

 Inflow Area =
 29,879 sf, 0.00% Impervious, Inflow Depth = 5.98" for RMAT 50-YR 24H TIER 3 event

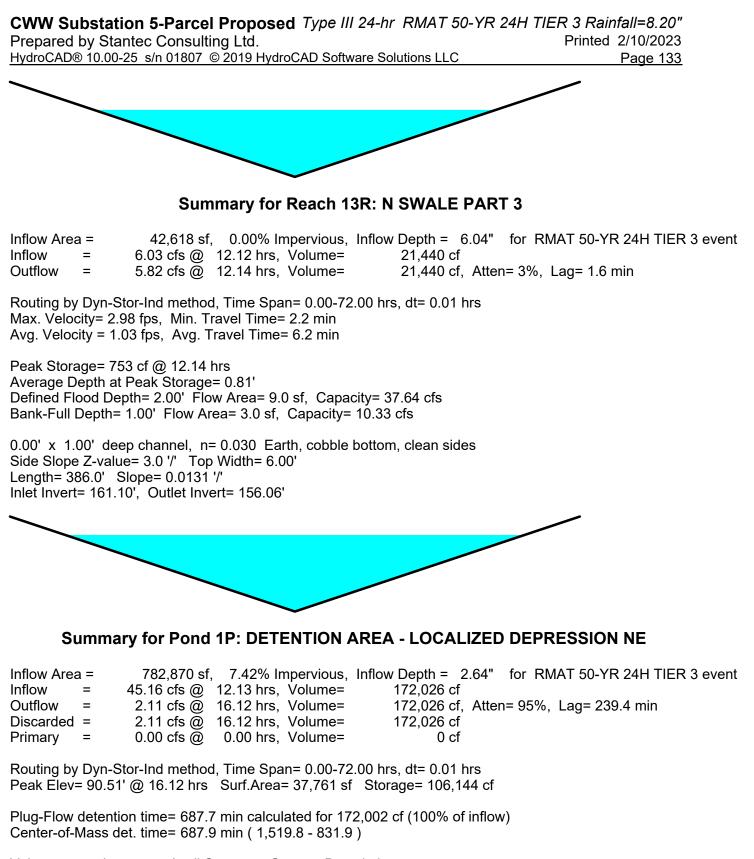
 Inflow =
 4.40 cfs @ 12.10 hrs, Volume=
 14,891 cf

 Outflow =
 4.17 cfs @ 12.13 hrs, Volume=
 14,891 cf, Atten= 5%, Lag= 1.9 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 2.80 fps, Min. Travel Time= 2.6 min Avg. Velocity = 0.99 fps, Avg. Travel Time= 7.3 min

Peak Storage= 643 cf @ 12.13 hrs Average Depth at Peak Storage= 0.70' Defined Flood Depth= 2.00' Flow Area= 9.0 sf, Capacity= 38.60 cfs Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 10.59 cfs

0.00' x 1.00' deep channel, n= 0.030 Earth, cobble bottom, clean sides Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 431.0' Slope= 0.0137 '/' Inlet Invert= 167.10', Outlet Invert= 161.18'



Volume	Invert	Avail.Storage	Storage Description
#1	83.00'	125,742 cf	Custom Stage Data (Irregular)Listed below (Recalc)

CWW Substation 5-Parcel Proposed *Type III 24-hr RMAT 50-YR 24H TIER 3 Rainfall=8.20"* Prepared by Stantec Consulting Ltd. Printed 2/10/2023

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Page 134

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>
83.00	827	135.0	0	0	827
84.00	2,756	217.0	1,698	1,698	3,131
85.00	5,016	298.0	3,830	5,528	6,460
86.00	8,057	383.0	6,477	12,004	11,079
87.00	12,347	488.0	10,126	22,130	18,370
88.00	17,968	589.0	15,070	37,200	27,043
89.00	25,096	690.0	21,433	58,633	37,342
90.00	33,858	865.0	29,368	88,001	59,011
91.00	41,761	936.1	37,740	125,742	69,242

Device	Routing	Invert	Outlet Devices
#1	Discarded	83.00'	2.410 in/hr Exfiltration over Surface area
#2	Primary	90.60'	50.0' long x 4.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66

2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32

Discarded OutFlow Max=2.11 cfs @ 16.12 hrs HW=90.51' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 2.11 cfs)

-1=Exhitration (Exhitration Controls 2.11 cis)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=83.00' TW=0.00' (Dynamic Tailwater) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 2P: SEDIMENT FOREBAY

Inflow Area =	168,079 sf, 0.00% Impervious,	Inflow Depth = 3.65" for RMAT 50-YR 24H TIER 3 event
Inflow =	13.08 cfs @ 12.15 hrs, Volume=	51,110 cf
Outflow =	12.94 cfs @ 12.17 hrs, Volume=	51,111 cf, Atten= 1%, Lag= 1.1 min
Discarded =	0.14 cfs @ 12.17 hrs, Volume=	16,787 cf
Primary =	12.80 cfs @ 12.17 hrs, Volume=	34,324 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 150.39' @ 12.17 hrs Surf.Area= 2,463 sf Storage= 9,783 cf Flood Elev= 151.00' Surf.Area= 2,531 sf Storage= 11,301 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 235.1 min (1,070.4 - 835.4)

Volume	Invert	: Avail.Sto	rage Storage	Description		
#1	146.00	22,3	48 cf Custon	n Stage Data (Co	nic)Listed below	(Recalc)
Elevatio (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
146.0 155.0	-	2,000 3,000	0 22,348	0 22,348	2,000 3,876	
Device	Routing	Invert	Outlet Device	es		
#1 #2	Discarded Primary	146.00' 150.00'	-	xfiltration over S 2.0' breadth Bro		angular Weir

Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Discarded OutFlow Max=0.14 cfs @ 12.17 hrs HW=150.39' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.14 cfs)

Primary OutFlow Max=12.79 cfs @ 12.17 hrs HW=150.39' TW=145.38' (Dynamic Tailwater) —2=Broad-Crested Rectangular Weir (Weir Controls 12.79 cfs @ 1.63 fps)

Summary for Pond 3P: DMHs w/ Vortex Unit

 Inflow Area =
 329,159 sf, 17.65% Impervious, Inflow Depth = 4.55" for RMAT 50-YR 24H TIER 3 event

 Inflow =
 37.29 cfs @
 12.09 hrs, Volume=
 124,842 cf

 Outflow =
 37.29 cfs @
 12.09 hrs, Volume=
 124,842 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 37.29 cfs @
 12.09 hrs, Volume=
 124,842 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 150.69' @ 12.09 hrs Flood Elev= 151.05'

Device	Routing	Invert	Outlet Devices
#1	Primary	148.00'	36.0" Round Culvert L= 25.0' Ke= 0.500 Inlet / Outlet Invert= 148.00' / 145.00' S= 0.1200 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf

Primary OutFlow Max=37.29 cfs @ 12.09 hrs HW=150.69' TW=145.36' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 37.29 cfs @ 5.58 fps)

Summary for Pond 4P: DMH

 Inflow Area =
 329,159 sf, 17.65% Impervious, Inflow Depth = 4.55" for RMAT 50-YR 24H TIER 3 event

 Inflow =
 37.29 cfs @
 12.09 hrs, Volume=
 124,842 cf

 Outflow =
 37.29 cfs @
 12.09 hrs, Volume=
 124,842 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 37.29 cfs @
 12.09 hrs, Volume=
 124,842 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 152.99' @ 12.09 hrs Flood Elev= 158.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	150.30'	36.0" Round Culvert L= 36.0' Ke= 0.500 Inlet / Outlet Invert= 150.30' / 148.00' S= 0.0639 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 7.07 sf

Summary for Pond 5P: DMH

Inflow Area = 216,616 sf, 16.26% Impervious, Inflow Depth = 4.50" for RMAT 50-YR 24H TIER 3 event 23.40 cfs @ 12.10 hrs, Volume= Inflow 81.205 cf = 23.40 cfs @ 12.10 hrs, Volume= Outflow = 81,205 cf, Atten= 0%, Lag= 0.0 min 23.40 cfs @ 12.10 hrs, Volume= Primary = 81.205 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 158.79' @ 12.10 hrs Flood Elev= 161.58' Device Routing Invert Outlet Devices

rtouting	Involu	Callet Devices
Primary	155.40'	24.0" Round Culvert L= 263.0' Ke= 0.500
-		Inlet / Outlet Invert= 155.40' / 150.50' S= 0.0186 '/' Cc= 0.900
		n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=23.38 cfs @ 12.10 hrs HW=158.79' TW=152.97' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 23.38 cfs @ 7.44 fps)

Summary for Pond 6P: OIL/WATER SEPARATOR

Inflow Area	a =	13,317 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	2.55 cfs @ 12.07 hrs, Volume= 8,834 cf
Outflow	=	2.55 cfs @ 12.07 hrs, Volume= 8,834 cf, Atten= 0%, Lag= 0.0 min
Primary	=	2.55 cfs @ 12.07 hrs, Volume= 8,834 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 158.82' @ 12.11 hrs Flood Elev= 162.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	155.90'	24.0" Round Culvert L= 27.0' Ke= 0.500 Inlet / Outlet Invert= 155.90' / 155.50' S= 0.0148 '/' Cc= 0.900 n= 0.012, Flow Area= 3.14 sf

Primary OutFlow Max=0.00 cfs @ 12.07 hrs HW=158.42' TW=158.57' (Dynamic Tailwater) -1=Culvert (Controls 0.00 cfs)

Summary for Pond 7P: DMH

13,317 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event Inflow Area = Inflow 2.55 cfs @ 12.07 hrs, Volume= 8,834 cf = 2.55 cfs @ 12.07 hrs, Volume= 8,834 cf, Atten= 0%, Lag= 0.0 min Outflow = Primary 2.55 cfs @ 12.07 hrs, Volume= 8,834 cf = Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.15' @ 12.11 hrs Flood Elev= 162.77' Dovice Douting Invert Outlet Devices

Device	Rouling	IIIVEIL	Outlet Devices
#1	Primary	156.40'	12.0" Round Culvert L= 40.0' Ke= 0.500

Inlet / Outlet Invert= 156.40' / 156.00' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.86 cfs @ 12.07 hrs HW=158.66' TW=158.42' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.86 cfs @ 2.36 fps)

Summary for Pond 8P: DMH

Inflow Area =	8,878 sf,100.00% Impervious,	Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow =	1.70 cfs @ 12.07 hrs, Volume=	5,889 cf
Outflow =	1.70 cfs @ 12.07 hrs, Volume=	5,889 cf, Atten= 0%, Lag= 0.0 min
Primary =	1.70 cfs @ 12.07 hrs, Volume=	5,889 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.41' @ 12.12 hrs Flood Elev= 164.88'

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 157.50'
 12.0" Round Culvert L= 154.0' Ke= 0.500 Inlet / Outlet Invert= 157.50' / 156.50' S= 0.0065 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.90 cfs @ 12.07 hrs HW=158.75' TW=158.66' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.90 cfs @ 1.18 fps)

Summary for Pond 9P: DMH

Inflow Area	a =	4,439 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.85 cfs @ 12.07 hrs, Volume= 2,945 cf
Outflow	=	0.85 cfs @ 12.07 hrs, Volume= 2,945 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.85 cfs @ 12.07 hrs, Volume= 2,945 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.85' @ 12.12 hrs Flood Elev= 166.83'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.30'	12.0" Round Culvert L= 141.0' Ke= 0.500 Inlet / Outlet Invert= 159.30' / 158.00' S= 0.0092 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.83 cfs @ 12.07 hrs HW=159.77' TW=158.75' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.83 cfs @ 3.33 fps)

Summary for Pond 10P: 24" Petro-Barrier

Inflow Area	a =	1,745 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.33 cfs @ 12.07 hrs, Volume= 1,158 cf
Outflow	=	0.33 cfs @ 12.07 hrs, Volume= 1,158 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.33 cfs @ 12.07 hrs, Volume= 1,158 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.81' @ 12.07 hrs Flood Elev= 169.43'

Device	Routing	Invert	Outlet Devices
#1	Primary	162.43'	6.0" Round Culvert L= 30.0' Ke= 0.500 Inlet / Outlet Invert= 162.43' / 162.00' S= 0.0143 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.33 cfs @ 12.07 hrs HW=162.81' TW=159.77' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.33 cfs @ 2.09 fps)

Summary for Pond 11P: 24" Petro-Barrier

Inflow Area	a =	2,694 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.52 cfs @ 12.07 hrs, Volume= 1,787 cf
Outflow	=	0.52 cfs @ 12.07 hrs, Volume= 1,787 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.52 cfs @ 12.07 hrs, Volume= 1,787 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 160.17' @ 12.07 hrs Flood Elev= 168.62'

DeviceRoutingInvertOutlet Devices#1Primary159.62'6.0" Round Culvert L= 18.0' Ke= 0.500
Inlet / Outlet Invert= 159.62' / 159.40' S= 0.0122 '/' Cc= 0.900
n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.52 cfs @ 12.07 hrs HW=160.17' TW=159.77' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.52 cfs @ 2.63 fps)

Summary for Pond 12P: 24" Petro-Barrier

Inflow Are	a =	1,745 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.33 cfs @ 12.07 hrs, Volume= 1,158 cf
Outflow	=	0.33 cfs @ 12.07 hrs, Volume= 1,158 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.33 cfs @ 12.07 hrs, Volume= 1,158 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 161.08' @ 12.07 hrs Flood Elev= 167.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	160.70'	6.0" Round Culvert L= 32.0' Ke= 0.500 Inlet / Outlet Invert= 160.70' / 160.40' S= 0.0094 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.33 cfs @ 12.07 hrs HW=161.08' TW=158.75' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.33 cfs @ 2.09 fps)

Summary for Pond 13P: 24" Petro-Barrier

Inflow Area = 2,694 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event Inflow 0.52 cfs @ 12.07 hrs. Volume= 1.787 cf = 0.52 cfs @ 12.07 hrs, Volume= Outflow = 1,787 cf, Atten= 0%, Lag= 0.0 min 0.52 cfs @ 12.07 hrs, Volume= Primary = 1,787 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.60' @ 12.12 hrs Flood Elev= 166.83' Device Routing Invert Outlet Devices 6.0" Round Culvert L= 20.0' Ke= 0.500 #1 157.83' Primary Inlet / Outlet Invert= 157.83' / 157.60' S= 0.0115 '/' Cc= 0.900

n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.33 cfs @ 12.07 hrs HW=158.87' TW=158.75' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 0.33 cfs @ 1.66 fps)

Summary for Pond 14P: 24" Petro-Barrier

Inflow Area	a =	2,694 sf,100.00% Impervious	, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.52 cfs @ 12.07 hrs, Volume=	1,787 cf
Outflow	=	0.52 cfs @ 12.07 hrs, Volume=	1,787 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.52 cfs @ 12.07 hrs, Volume=	1,787 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.36' @ 12.12 hrs Flood Elev= 165.88'

Device	Routing	Invert	Outlet Devices
#1	Primary	156.88'	6.0" Round Culvert L= 21.0' Ke= 0.500 Inlet / Outlet Invert= 156.88' / 156.60' S= 0.0133 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.23 cfs @ 12.07 hrs HW=158.72' TW=158.66' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.23 cfs @ 1.17 fps)

Summary for Pond 15P: 24" Petro-Barrier

Inflow Area = 1,745 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event Inflow = 0.33 cfs @ 12.07 hrs, Volume= 1,158 cf Outflow = 0.33 cfs @ 12.07 hrs, Volume= 1,158 cf, Atten= 0%, Lag= 0.0 min Primary = 0.33 cfs @ 12.07 hrs, Volume= 1,158 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Peak Elev= 159.25' @ 12.12 hrs Flood Elev= 164.66'

Device	Routing	Invert	Outlet Devices
#1	Primary	157.66'	6.0" Round Culvert L= 34.0' Ke= 0.500

Inlet / Outlet Invert= 157.66' / 157.40' S= 0.0076 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.00 cfs @ 12.07 hrs HW=158.56' TW=158.66' (Dynamic Tailwater)

Summary for Pond 16P: DMH

Inflow Area =	52,511 sf, 0.00% Impervious,	Inflow Depth = 3.83" for RMAT 50-YR 24H TIER 3 event
Inflow =	5.40 cfs @ 12.09 hrs, Volume=	16,745 cf
Outflow =	5.40 cfs @ 12.09 hrs, Volume=	16,745 cf, Atten= 0%, Lag= 0.0 min
Primary =	5.40 cfs $\overline{@}$ 12.09 hrs, Volume=	16,745 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 161.54' @ 12.09 hrs Flood Elev= 162.19'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.00'	12.0" Round Culvert L= 17.0' Ke= 0.500 Inlet / Outlet Invert= 159.00' / 158.50' S= 0.0294 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=5.39 cfs @ 12.09 hrs HW=161.53' TW=158.77' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 5.39 cfs @ 6.87 fps)

Summary for Pond 17P: DMH

Inflow Area	a =	150,788 sf, 14.52% Impervious, Inflow Depth = 4.43" for RMAT 50-YR 24H TIER 3 event
Inflow	=	15.74 cfs @ 12.11 hrs, Volume= 55,627 cf
Outflow	=	15.74 cfs @ 12.11 hrs, Volume= 55,627 cf, Atten= 0%, Lag= 0.0 min
Primary	=	15.74 cfs @ 12.11 hrs, Volume= 55,627 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 161.48' @ 12.11 hrs Flood Elev= 163.59'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.40'	24.0" Round Culvert L= 88.2' Ke= 0.500 Inlet / Outlet Invert= 159.40' / 155.50' S= 0.0442 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=15.72 cfs @ 12.11 hrs HW=161.48' TW=158.76' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 15.72 cfs @ 5.01 fps)

Summary for Pond 18P: DMH

Inflow Area	=	128,888 sf,	0.00% Impervious,	Inflow Depth =	3.83"	for RMAT 5	0-YR 24H TIER 3 event
Inflow =	=	12.14 cfs @	12.12 hrs, Volume=	41,100 c	f		
Outflow =	=	12.14 cfs @	12.12 hrs, Volume=	41,100 c	f, Atter	= 0%, Lag=	0.0 min
Primary =	=	12.14 cfs @	12.12 hrs, Volume=	41,100 c	f	-	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 164.29' @ 12.12 hrs Flood Elev= 165.39'

Device	Routing	Invert	Outlet Devices
#1	Primary	161.50'	18.0" Round Culvert L= 92.0' Ke= 0.500 Inlet / Outlet Invert= 161.50' / 159.50' S= 0.0217 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=12.13 cfs @ 12.12 hrs HW=164.28' TW=161.46' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 12.13 cfs @ 6.86 fps)

Summary for Pond 20P: DMH

Inflow Area	a =	102,433 sf, 12.46% Impervious, Inflow Depth = 4.33" for RMAT 50-YR 24H TIER 3 e	vent
Inflow	=	12.15 cfs @ 12.08 hrs, Volume= 36,931 cf	
Outflow	=	12.15 cfs @ 12.08 hrs, Volume= 36,931 cf, Atten= 0%, Lag= 0.0 min	
Primary	=	12.15 cfs @ 12.08 hrs, Volume= 36,931 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.81' @ 12.09 hrs Flood Elev= 158.03'

DeviceRoutingInvertOutlet Devices#1Primary152.00'24.0" Round Culvert L= 56.0' Ke= 0.500
Inlet / Outlet Invert= 152.00' / 150.40' S= 0.0286 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=11.88 cfs @ 12.08 hrs HW=153.80' TW=152.97' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 11.88 cfs @ 5.28 fps)

Summary for Pond 21P: DMH

Inflow Are	a =	25,304 sf, 16.82% Impervious, Inflow Depth = 4.52" for RMAT 50-YR 24H TIER 3 event
Inflow	=	3.19 cfs @ 12.07 hrs, Volume= 9,531 cf
Outflow	=	3.19 cfs @ 12.07 hrs, Volume= 9,531 cf, Atten= 0%, Lag= 0.0 min
Primary	=	3.19 cfs @ 12.07 hrs, Volume= 9,531 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 156.15' @ 12.07 hrs Flood Elev= 157.94'

Device	Routing	Invert	Outlet Devices
#1	Primary	154.94'	12.0" Round Culvert L= 62.0' Ke= 0.500 Inlet / Outlet Invert= 154.94' / 152.40' S= 0.0410 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=3.19 cfs @ 12.07 hrs HW=156.15' TW=153.77' (Dynamic Tailwater) -1=Culvert (Inlet Controls 3.19 cfs @ 4.06 fps)

Summary for Pond 22P: DMH

Inflow Area = 77,129 sf, 11.04% Impervious, Inflow Depth = 4.26" for RMAT 50-YR 24H TIER 3 event Inflow 8.97 cfs @ 12.08 hrs. Volume= 27.399 cf = 8.97 cfs @ 12.08 hrs, Volume= Outflow = 27,399 cf, Atten= 0%, Lag= 0.0 min 8.97 cfs @ 12.08 hrs, Volume= Primary = 27,399 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 157.36' @ 12.08 hrs Flood Elev= 158.66' Device Routing Invert Outlet Devices #1 155.50' 18.0" Round Culvert L= 56.0' Ke= 0.500 Primary Inlet / Outlet Invert= 155.50' / 155.00' S= 0.0089 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Summary for Pond 23P: OIL/WATER SEPARATOR

Inflow Area	=	10,110 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	1.94 cfs @ 12.07 hrs, Volume= 6,706 cf
Outflow	=	1.98 cfs @ 12.07 hrs, Volume= 6,706 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.98 cfs @ 12.07 hrs, Volume= 6,706 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.49' @ 12.09 hrs Flood Elev= 159.18'

Device	Routing	Invert	Outlet Devices
#1	Primary	141.80'	12.0" Round Culvert L= 182.0' Ke= 0.500 Inlet / Outlet Invert= 141.80' / 140.10' S= 0.0093 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf
			n= 0.011 Concrete pipe, straight & clean, 110w Area- 0.79 st

Primary OutFlow Max=1.84 cfs @ 12.07 hrs HW=153.41' TW=152.93' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 1.84 cfs @ 2.34 fps)

Summary for Pond 24P: DMH

 Inflow Area =
 10,110 sf,100.00% Impervious, Inflow Depth =
 7.96" for RMAT 50-YR 24H TIER 3 event

 Inflow =
 1.94 cfs @
 12.07 hrs, Volume=
 6,706 cf

 Outflow =
 1.94 cfs @
 12.07 hrs, Volume=
 6,706 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 1.94 cfs @
 12.07 hrs, Volume=
 6,706 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.71' @ 12.10 hrs Flood Elev= 159.61'

Device	Routing	Invert	Outlet Devices
#1	Primary	150.20'	12.0" Round Culvert L= 1.0' Ke= 0.500

Inlet / Outlet Invert= 150.20' / 150.10' S= 0.1000 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.29 cfs @ 12.07 hrs HW=153.52' TW=153.41' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.29 cfs @ 1.65 fps)

Summary for Pond 25P: 24" Petro-Barrier

Inflow Area =	1,770 sf,100.00% Impervious,	Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow =	0.34 cfs @ 12.07 hrs, Volume=	1,174 cf
Outflow =	0.34 cfs @ 12.07 hrs, Volume=	1,174 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.34 cfs @ 12.07 hrs, Volume=	1,174 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.87' @ 12.11 hrs Flood Elev= 159.89'

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 150.89'
 6.0" Round Culvert L= 58.0' Ke= 0.500 Inlet / Outlet Invert= 150.89' / 150.30' S= 0.0102 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.23 cfs @ 12.07 hrs HW=153.62' TW=153.52' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.23 cfs @ 1.19 fps)

Summary for Pond 26P: 24" Petro-Barrier

Inflow Area	a =	1,770 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.34 cfs @ 12.07 hrs, Volume= 1,174 cf
Outflow	=	0.34 cfs @ 12.07 hrs, Volume= 1,174 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.34 cfs @ 12.07 hrs, Volume= 1,174 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 153.82' @ 12.11 hrs Flood Elev= 161.98'

Device	Routing	Invert	Outlet Devices
#1	Primary	152.98'	6.0" Round Culvert L= 17.0' Ke= 0.500 Inlet / Outlet Invert= 152.98' / 152.00' S= 0.0576 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.15 cfs @ 12.07 hrs HW=153.55' TW=153.52' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.15 cfs @ 0.79 fps)

Summary for Pond 27P: DMH

Inflow Area	a =	6,570 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	1.26 cfs @ 12.07 hrs, Volume= 4,358 cf
Outflow	=	1.26 cfs @ 12.07 hrs, Volume= 4,358 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.26 cfs @ 12.07 hrs, Volume= 4,358 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 154.58' @ 12.09 hrs Flood Elev= 162.35'

Device	Routing	Invert	Outlet Devices
-	Primary	153.90'	12.0" Round Culvert L= 224.0' Ke= 0.500 Inlet / Outlet Invert= 153.90' / 152.00' S= 0.0085 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.20 cfs @ 12.07 hrs HW=154.55' TW=153.52' (Dynamic Tailwater) -1=Culvert (Outlet Controls 1.20 cfs @ 3.15 fps)

Summary for Pond 28P: DMH

Inflow Area	=	6,570 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	1.26 cfs @ 12.07 hrs, Volume= 4,358 cf
Outflow	=	1.26 cfs @ 12.07 hrs, Volume= 4,358 cf, Atten= 0%, Lag= 0.0 min
Primary	=	1.26 cfs @ 12.07 hrs, Volume= 4,358 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 155.03' @ 12.08 hrs Flood Elev= 163.41'

DeviceRoutingInvertOutlet Devices#1Primary154.40'**12.0" Round Culvert** L= 50.0' Ke= 0.500
Inlet / Outlet Invert= 154.40' / 154.00' S= 0.0080 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.22 cfs @ 12.07 hrs HW=155.02' TW=154.55' (Dynamic Tailwater) -1=Culvert (Outlet Controls 1.22 cfs @ 3.37 fps)

Summary for Pond 29P: 24" Petro-Barrier

 Inflow Area =
 1,770 sf,100.00% Impervious, Inflow Depth =
 7.96" for RMAT 50-YR 24H TIER 3 event

 Inflow =
 0.34 cfs @
 12.07 hrs, Volume=
 1,174 cf

 Outflow =
 0.34 cfs @
 12.07 hrs, Volume=
 1,174 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.34 cfs @
 12.07 hrs, Volume=
 1,174 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 155.33' @ 12.07 hrs Flood Elev= 163.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	154.95'	6.0" Round Culvert L= 7.0' Ke= 0.500 Inlet / Outlet Invert= 154.95' / 154.50' S= 0.0643 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.34 cfs @ 12.07 hrs HW=155.33' TW=155.02' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.34 cfs @ 2.10 fps)

Summary for Pond 30P: DMH

Inflow Area = 4,800 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event Inflow 0.92 cfs @ 12.07 hrs. Volume= 3.184 cf = 0.92 cfs @ 12.07 hrs, Volume= Outflow = 3,184 cf, Atten= 0%, Lag= 0.0 min 0.92 cfs @ 12.07 hrs, Volume= Primary = 3,184 cf Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 160.89' @ 12.07 hrs Flood Elev= 165.34' Device Routing Invert Outlet Devices #1 160.40' 12.0" Round Culvert L= 98.0' Ke= 0.500 Primary Inlet / Outlet Invert= 160.40' / 159.50' S= 0.0092 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.92 cfs @ 12.07 hrs HW=160.89' TW=155.02' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.92 cfs @ 2.39 fps)

Summary for Pond 31P: DMH

 Inflow Area =
 4,800 sf,100.00% Impervious, Inflow Depth =
 7.96" for RMAT 50-YR 24H TIER 3 event

 Inflow =
 0.92 cfs @
 12.07 hrs, Volume=
 3,184 cf

 Outflow =
 0.92 cfs @
 12.07 hrs, Volume=
 3,184 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.92 cfs @
 12.07 hrs, Volume=
 3,184 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.40' @ 12.07 hrs Flood Elev= 168.53'

Device	Routing	Invert	Outlet Devices
#1	Primary	161.90'	12.0" Round Culvert L= 232.0' Ke= 0.500 Inlet / Outlet Invert= 161.90' / 160.50' S= 0.0060 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.92 cfs @ 12.07 hrs HW=162.40' TW=160.89' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.92 cfs @ 3.41 fps)

Summary for Pond 32P: 24" Petro-Barrier

 Inflow Area =
 1,600 sf,100.00% Impervious, Inflow Depth =
 7.96" for RMAT 50-YR 24H TIER 3 event

 Inflow =
 0.31 cfs @
 12.07 hrs, Volume=
 1,061 cf

 Outflow =
 0.31 cfs @
 12.07 hrs, Volume=
 1,061 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.31 cfs @
 12.07 hrs, Volume=
 1,061 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 162.80' @ 12.07 hrs Flood Elev= 170.69'

Device	Routing	Invert	Outlet Devices
#1	Primary	162.44'	6.0" Round Culvert L= 10.0' Ke= 0.500

Inlet / Outlet Invert= 162.44' / 162.20' S= 0.0240 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.31 cfs @ 12.07 hrs HW=162.80' TW=162.40' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.31 cfs @ 2.04 fps)

Summary for Pond 33P: DMH

Inflow Area =	3,200 sf,100.00% Impervious,	Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow =	0.61 cfs @ 12.07 hrs, Volume=	2,123 cf
Outflow =	0.61 cfs @ 12.07 hrs, Volume=	2,123 cf, Atten= 0%, Lag= 0.0 min
Primary =	0.61 cfs $\overline{@}$ 12.07 hrs, Volume=	2,123 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 163.59' @ 12.07 hrs Flood Elev= 170.43'

DeviceRoutingInvertOutlet Devices#1Primary163.20'**12.0" Round Culvert** L= 159.0' Ke= 0.500
Inlet / Outlet Invert= 163.20' / 162.00' S= 0.0075 '/' Cc= 0.900
n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.61 cfs @ 12.07 hrs HW=163.59' TW=162.40' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.61 cfs @ 2.14 fps)

Summary for Pond 34P: 24" Petro-Barrier

Inflow Area	a =	1,600 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.31 cfs @ 12.07 hrs, Volume= 1,061 cf
Outflow	=	0.31 cfs @ 12.07 hrs, Volume= 1,061 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.31 cfs @ 12.07 hrs, Volume= 1,061 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 163.80' @ 12.07 hrs Flood Elev= 171.68'

Device	Routing	Invert	Outlet Devices
#1	Primary	163.43'	6.0" Round Culvert L= 8.0' Ke= 0.500 Inlet / Outlet Invert= 163.43' / 163.30' S= 0.0162 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.30 cfs @ 12.07 hrs HW=163.80' TW=163.59' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.30 cfs @ 2.70 fps)

Summary for Pond 35P: 24" Petro-Barrier

Inflow Area	a =	1,600 sf,100.00% Impervious, Inflow Depth = 7.96" for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.31 cfs @ 12.07 hrs, Volume= 1,061 cf
Outflow	=	0.31 cfs @ 12.07 hrs, Volume= 1,061 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.31 cfs @ 12.07 hrs, Volume= 1,061 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 164.75' @ 12.07 hrs Flood Elev= 172.64'

Device	Routing	Invert	Outlet Devices
	Primary	164.39'	6.0" Round Culvert L= 38.0' Ke= 0.500 Inlet / Outlet Invert= 164.39' / 164.00' S= 0.0103 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.31 cfs @ 12.07 hrs HW=164.75' TW=163.59' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.31 cfs @ 2.04 fps)

Summary for Pond 40P: DMH

Inflow Area =	105,170 sf, 0.00% Impervious,	Inflow Depth = 2.65" for RMAT 50-YR 24H TIER 3 event
Inflow =	5.72 cfs @ 12.13 hrs, Volume=	23,199 cf
Outflow =	5.72 cfs @ 12.13 hrs, Volume=	23,199 cf, Atten= 0%, Lag= 0.0 min
Primary =	5.72 cfs @ 12.13 hrs, Volume=	23,199 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.08' @ 12.14 hrs Flood Elev= 160.03'

Device	Routing	Invert	Outlet Devices
#1	Primary	158.00'	24.0" W x 12.0" H Box Culvert L= 10.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 158.00' / 157.70' S= 0.0300 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 2.00 sf

Primary OutFlow Max=5.66 cfs @ 12.13 hrs HW=159.07' TW=158.66' (Dynamic Tailwater) -1=Culvert (Inlet Controls 5.66 cfs @ 2.83 fps)

Summary for Pond 41P: CULVERT

Inflow Area =	87,839 sf, 0.00% Impervious, Inflow Depth = 2.07" for RMAT 50-YR 24H TIER 3 event
Inflow =	3.83 cfs @ 12.16 hrs, Volume= 15,146 cf
Outflow =	3.83 cfs @ 12.16 hrs, Volume= 15,146 cf, Atten= 0%, Lag= 0.0 min
Primary =	3.83 cfs @ 12.16 hrs, Volume= 15,146 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 159.71' @ 12.16 hrs Flood Elev= 161.08'

Device	Routing	Invert	Outlet Devices
#1	Primary	159.00'	24.0" W x 12.0" H Box Culvert L= 55.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 159.00' / 158.00' S= 0.0182 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 2.00 sf

Primary OutFlow Max=3.83 cfs @ 12.16 hrs HW=159.71' TW=159.06' (Dynamic Tailwater) -1=Culvert (Inlet Controls 3.83 cfs @ 2.70 fps)

Summary for Link 1L: OVERFLOW

Inflow Are	a =	782,870 sf,	7.42% Impervious,	Inflow Depth = 0.00"	for RMAT 50-YR 24H TIER 3 event
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Summary for Link 10L: OFF-SITE

Inflow Area =	258,650 sf, 0.00% Impervious, Inflow Depth = 0.62" for RMAT 50-YR 24H TIER 3 event
Inflow =	1.49 cfs @ 12.32 hrs, Volume= 13,345 cf
Primary =	1.49 cfs @ 12.32 hrs, Volume= 13,345 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Section 3

Erosion & Sediment Control Plan

Erosion & Sedimentation Control Plan for the Stormwater Management Plan for 275/345KV Substation for New England Wind 2 Connector Project

Prepared for:

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Prepared by:

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Project No. 198804104

February 10, 2023

TABLE OF CONTENTS

				<u>Page</u>	
1.0	Plan	Objectiv	es	3	
2.0	Introc	luction		3	
3.0	Curre	ent Site (Conditions	3	
4.0	Proje	ct Descr	iption	4	
	4.1	Propo	sed Site Conditions	4	
5.0	Erosi	on & Se	dimentation Control Plan	5	
	5.1	Major	Const. Sequence for Site	5	
	5.2	Best N	lanagement Practices	6	
		5.2.1	Dumpster	6	
		5.2.2	Erosion Control Barrier	6	
		5.2.3	Dust Control	7	
		5.2.4	Disturbed Surface Maintenance	7	
		5.2.5	Temp. Stormwater Controls	7	

1.0 – Plan Objectives

- To protect abutting properties, public ways and drainage infrastructure from construction related pollutant impacts generated from land disturbance and construction activities;
- Control existing, and potential erosion, sediment transport and pollutant impact events by installing and maintaining construction related Best Management Practices (BMP's) to reduce and/or prevent the discharge of stormwater pollutants into groundwater of the Commonwealth of Massachusetts;
- To protect surface stormwater quality, ground water quality, and minimize off-site sediment transport during construction;
- To prevent local and off-site flooding by controlling peak rates and volumes of stormwater runoff during construction; and
- To eliminate illicit discharges to stormwater drainage systems that may cause pollution during construction.

2.0 – Introduction

This Erosion and Sedimentation Control Plan (The "Plan") has been devised for the construction of a new substation consisting of gravel roadways, crushed stone surfacing, buildings, and electrical equipment for the proposed 275/245KV Substation for the New England Wind 2 Connector project.

The purpose of the Plan is to protect the surrounding environment from sediment-laden stormwater during construction of the electrical equipment and supporting infrastructure. The stormwater will be treated before release or infiltration, and surfaces stabilized to minimize erosive events by implementing, installing and maintaining construction related Best Management Practices (BMP's). These BMPs will reduce and/or prevent the discharge of stormwater pollutants into groundwater resources of the Commonwealth of Massachusetts. The BMP's are described in the MassDEP Stormwater Policy Manual as developed by the Massachusetts Department for Environmental Protection (MassDEP) and it is our belief that short-term construction related pollution prevention generated from this Site can be achieved.

3.0 - Current Site Conditions

The proposed project site is comprised of four adjacent properties in Barnstable, Massachusetts. Property areas, from west to east, are 5.4 acres, 7.3 acres, 7.5 acres and 3.4 acres. All 4 properties are currently entirely forested, with the exception of a small 'panhandle' spur in the north of the center property that is partially occupied by a clearcut electrical easement. The proposed substation will occupy a portion of the areas of the properties, primarily in the south and center. An existing access road, leading from Oak Street to the fire tower at Clay Hill, passes the southern boundary of the three easternmost properties and will be connected to the proposed substation. An Existing Conditions Tributary Area Plan is attached to the Stormwater Management Report. SCS Method¹ CN and time of concentration values were calculated to determine the peak runoff rates and volumes for each existing sub-catchment area.

The highest elevation at the Site is approximately 195' above mean sea level (msl) in the south of the site, while the lowest elevation is approximately 83' in the north-center of the site. Generally, much of the site topography slopes from the south to the low point in the north, although smaller sub-catchments are present along the boundaries of the site (see the Existing Conditions Plan, attached to the Stormwater Management Report, for more details).

4.0 - Project Description

4.1 - Proposed Project

The proposed project includes the construction of a new substation with electrical equipment, buildings, crushed stone surfaced area, gravel ring road and access road, and a paved driveway apron. The proposed project will also require significant re-grading and retaining structures in order to maintain a grade of no greater than 2 percent within the substation yard. The primary purpose of this project is to build a substation that will allow transmission of electrical energy while allowing safe and secure access to the site by authorized personnel.

The new electrical equipment area, and interior roads will be graded such that stormwater is directed to perforated drains beneath crushed rock surfacing on the site, and then contained on site in a detention/infiltration basin. For a complete description of the stormwater management and drainage facilities, refer to the Stormwater Management Plan.

The lot which is to be developed is already well vegetated with native species, however a landscape buffer will be maintained on the boundaries to the extent feasible so that some native woodland is preserved around the site.

All areas disturbed by construction activity shall be stabilized, and either paved, or covered with crushed stone, or planted and maintained. During site work construction, there will be a soil stockpile area and one temporary sediment basin at the northern end of the Site. The specific location, size, and design of these areas will be submitted as part of final Site design plans at a later date.

¹ Soil Conservation Service hydrologic method TR-55 was used to develop the Curve Number (CN) and Time of Concentration (Tc) values used for hydrologic analysis of pre-and post-development stormwater runoff values.

5.0 - Erosion & Sedimentation Control Plan

The contractor shall implement an Erosion and Sedimentation Control Plan that protects the surrounding environment from sediment laden stormwater runoff generated during construction activities and from other pollutants generated from construction activities such as litter and dust. Construction sequencing is part of managing a site as is implementing BMP's that assist in controlling construction related stormwater and pollutants.

5.1 - Major Construction Sequence for Site Work

The following sequence has been developed to contain all potential sedimentation and erosion incidents that could occur during the construction of the project. The contractor however is responsible to manage the site effectively to control offsite sediment transport which may not be included in this plan. The sequence will coordinate the work within the erosion barrier and coordinate other sedimentation control features to reduce the stress upon a silt fence or other deployed sediment barrier as well as limit off-site sediment transport or entry of construction related sediment to any catch basin located at the Site. The sequencing is as follows:

- Clear and grub such that a construction safety fence can be immediately installed around the property, to limit access and protect the public.
- Place an erosion control or sediment control barrier (straw wattle) at the limit of work where possible
- Carry out re-grading and expansion works to the Clay Hill fire tower access road, which is to be used for construction and permanent access to the proposed substation.
- Have a water truck on-site and use as necessary to minimize fugitive dust during excavation, demolition of existing pavement surfaces and general construction processes.
- Clear and grub, and remove top soils, and excavate as necessary to create the temporary sedimentation basin (at northern end of the Site) as intended to manage stormwater during the construction. Please note the location and design of this temporary basin will be determined during the final design phase of the project.
- Grade the gravel subbase for areas where a finished crushed stone surface will be placed (i.e. for the electrical equipment yard). Install retaining walls on the northern and southern ends (including partial sides) of the Site.
- Finalize grading of the site's sand & gravel sub-base surface, which will become the sub-base elevations of the substation.
- Install the footings and foundations for the proposed electrical equipment, containment areas, building, etc.
- Install all drainage features including the infiltration basin, deep sump catch basins, a vortex water quality unit, and two oil-water separators.

- Construct the proposed ring road and associated parking areas.
- Finalize grading within the confines of the substation yard, including the double washed crushed stone surface
- Clean up the Site, remove silt sacks, clean catch basin(s), and remove siltation barriers, construction entrance apron, and construction limit fencing.

The contractor has several procedures to perform in order to maintain the site. They include but are not limited to:

- Replace erosion control barriers at limit of work as needed. Barrier to be inspected on a weekly basis.
- Empty silt sacks that have been installed at any relevant catch basins after each rain event. Catch basins and manholes are to be cleaned once sediment occupies 1/2 the sump available. Structures are to be inspected on a weekly basis.
- Stone apron used for construction egress is to be replaced as sediment builds up; and this apron is to be inspected on a daily basis.
- All stockpiled soils (topsoil, special structural fill, etc.) are to be covered to minimize fugitive dust.
- All exposed slopes are to be stabilized with erosion control netting and/or temporary plantings.
- Maintain a covered dumpster on site to minimize windblown debris from littering neighborhood and resource areas.
- Have a water truck onsite for use during the demolition portion of the project and during rough grading to provide water to minimize fugitive dust.

5.2 - Best Management Practices

The contractor shall employ various types of structural and non-structural methodologies to minimize offsite pollution from construction activities. The following is a list of some BMP's that can be utilized; however, it is the contractor's responsibility to implement their strategies to minimize offsite sediment transport and fugitive dust and trash:

5.2.1 - Dumpster

The contractor shall have a dumpster on-site for the disposal of construction debris. The contractor shall cover the dumpster as needed to prevent windblown debris from becoming litter in the environment.

5.2.2 - Erosion Control Barrier

An erosion control barrier, as detailed on the project plans, shall be installed at the downgradient Limits of Work and used around the site as needed. In addition, a barrier of the same type shall be used around soil stockpiles and localized excavations on site. The barrier needs to be effective in controlling sediment transport and shall not be allowed to become strained or stressed as the project moves forward. The contractor shall inspect the barrier weekly or after a large storm event to identify any stressed areas and replace the barrier as needed. The barrier can be one or many of several types. Staked haybales, straw wattles, or geotextile fabric or a geotextile erosion control sock are typically acceptable types of barriers; and these shall be backed up by silt fence material placed on the interior side of the proposed construction fence. The contractor shall inspect erosion control barriers daily and repair the barriers as needed.

5.2.3 - Dust Control

The use of a water truck or other method to spray water over the site shall be implemented during the dry season to minimize blown dust. The water shall not be excessively spread so erosive forces occur. The contractor shall sweep new pavement once installed and cover stockpiled soils as needed to minimize dust.

5.2.4 - Disturbed Surface Maintenance

The contractor shall stabilize the ground surface as needed to prevent erosion. Stabilization of surfaces includes the placement of pavement, crushed stone in yard areas, rip rap, erosion control netting, wood bark or haymulch, and the establishment of vegetated surfaces. Upon the completion of construction of a phase, all surfaces should be stabilized even though it is apparent that future construction efforts will cause their disturbance. Vegetated cover should be established during the proper growing season and should be enhanced by soil adjustment for proper pH, nutrients and moisture content. Surfaces that are disturbed by erosion processes or vandalism should be stabilized as soon as possible. Areas where construction activities have permanently or temporarily ceased should be stabilized within 14 days from the date of last construction activity, except when construction activity will resume within 21 days (e.g., the total time that construction activity is temporarily ceased is less than 21 days). Haybale dikes or silt fences should be set where required to trap products of erosion and should be maintained on a continuing basis during the construction process. Wheel ruts should be filled in and graded to prevent concentration of stormwater runoff. Vehicle tracks leading downhill should be blocked during periods of intense precipitation by haybales, dikes or silt fences which should be constructed to entrap the sediment.

5.2.5 - Temporary Stormwater Controls

Generally, if possible, the contractor shall rough grade the site so as to not concentrate the stormwater runoff and cause erosive forces. The contractor shall use a level spreader or other temporary stormwater control device to treat construction site runoff for suspended solids. Once final installation of the stormwater BMPs occurs, which should be after base course paving is completed, they will need to be cleaned of all construction sediment before hydraulically connecting them to each other. The use of silt sacks on deep sump catch basins will help minimize the cleaning of the sump. Temporary sediment basins installed to assist in capturing construction site runoff will need to be cleaned of all sediment by over-excavation before they are backfilled and properly compacted back to proposed finished grades. If any infiltration basin area or other future construction zones are to be used as temporary stormwater management basins, then these areas shall be re-excavated prior to final construction to a depth necessary to completely remove any accumulated sediments and silts or impacts on existing soil that might have resulted from such accumulations during construction.

Section 4

Operations and Maintenance Plan for Proposed Stormwater BMPs

(to be submitted during final design)

Section 5

Massachusetts Checklist for Stormwater Report



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.¹ This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

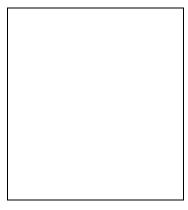
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature

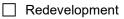


Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

New development



Mix of New Development and Redevelopment



LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- Reduced Impervious Area (Redevelopment Only)
- Minimizing disturbance to existing trees and shrubs
- LID Site Design Credit Requested:
 - Credit 1
 - Credit 2
 - Credit 3
- Use of "country drainage" versus curb and gutter conveyance and pipe
- Bioretention Cells (includes Rain Gardens)
- Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- Treebox Filter
- Water Quality Swale
- Grass Channel
- Green Roof
- Other (describe):

Standard 1: No New Untreated Discharges

- No new untreated discharges
- Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.

Calculations provided to show that post-development peak discharge rates do not exceed predevelopment rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24hour storm.

Standard 3: Recharge

Soil Analysis provided.

- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.

Static	Simple Dynamic
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Dynamic Field¹

- Runoff from all impervious areas at the site discharging to the infiltration BMP.
- Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
 - Site is comprised solely of C and D soils and/or bedrock at the land surface
 - M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
 - Solid Waste Landfill pursuant to 310 CMR 19.000
 - Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- Calculations showing that the infiltration BMPs will drain in 72 hours are provided.

	Property	[,] includes a	M.G.L. c.	21E site or	a solid waste	landfill and	a mounding ar	alysis is included.
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¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Standard 3: Recharge (continued)

The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.

Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
- Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - is within the Zone II or Interim Wellhead Protection Area
 - is near or to other critical areas
 - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - involves runoff from land uses with higher potential pollutant loads.
- The Required Water Quality Volume is reduced through use of the LID site Design Credits.
- Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



Sta	Standard 4: Water Quality (continued)					
\boxtimes	The BMP is sized (and calculations provided) based on:					
	The ½" or 1" Water Quality Volume or					
	The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.					
	The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.					
	A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.					
Sta	ndard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)					
	The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report. The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted prior to the discharge of stormwater to the post-construction stormwater BMPs.					
	The NPDES Multi-Sector General Permit does <i>not</i> cover the land use.					
	LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.					
	All exposure has been eliminated.					
	All exposure has <i>not</i> been eliminated and all BMPs selected are on MassDEP LUHPPL list.					
	The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.					
Sta	indard 6: Critical Areas					
	The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.					

Critical areas and BMPs are identified in the Stormwater Report.



Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

Limited Proje	ect
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- Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- Bike Path and/or Foot Path
- Redevelopment Project
- Redevelopment portion of mix of new and redevelopment.
- Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

The project is highly complex and information is included in the Stormwater Report that explains why
it is not possible to submit the Construction Period Pollution Prevention and Erosion and
Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and
Erosion and Sedimentation Control has <i>not</i> been included in the Stormwater Report but will be
submitted <i>before</i> land disturbance begins.

The project is <i>not</i> covered by a NPDES Construction General Perm
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- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

The Post Construction Operation and Maintenance Plan is included in the Stormwater Re	port and
includes the following information:	

- Name of the stormwater management system owners;
- Party responsible for operation and maintenance;
- Schedule for implementation of routine and non-routine maintenance tasks;
- Plan showing the location of all stormwater BMPs maintenance access areas;
- Description and delineation of public safety features;
- Estimated operation and maintenance budget; and
- Operation and Maintenance Log Form.
- The responsible party is *not* the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted *prior to* the discharge of any stormwater to post-construction BMPs.

Section 6

Nitrogen Loading Calculation



Nitrogen Loading Calculation December 16, 2022

Based on Cape Cod Commission Technical Bulletin 91-001 (April 1992)

Total Developed Area: 423,714 ft² Impervious Surfaces: Roof 34,667 ft², Concrete Containment 23,427 ft², Other Impervious areas 62,463 ft² Pervious Surfaces: 303,157 ft². Wastewater Flows: N/A

Wastewater – 0 L/d

Lawns – 0 L/d

Impervious Surfaces –

Roof area, concrete containments, and other impervious areas

120,557 ft² x $\left[\frac{40 \text{ in}}{yr}\right] \left[\frac{ft}{12 \text{ in}}\right] \left[\frac{28.32 \text{ L}}{ft^3}\right] \left[\frac{1 \text{ yr}}{365 \text{ d}}\right] = 31,179.7 \text{ L/d x} \left[\frac{0.75 \text{ mg}}{L}\right] = 23,384.8 \text{ mg/d}$

Pervious Surfaces –

303,157 ft² x $\left[\frac{18 in}{yr}\right] \left[\frac{ft}{12 in}\right] \left[\frac{28.32 L}{ft^3}\right] \left[\frac{1 yr}{365 d}\right] = 35,282.5 L/d$

Summary

 $\frac{23,384.8 \ mg}{31,179.7+35,282.5 \ L} = \mathbf{0.35 \ ppm}$

APPENDED DRAWING SHEETS

(See DEIR Attachment C2)

- SHEET 1 COVER SHEET
- SHEET 2 GENERAL NOTES
- SHEET 3 EXISTING CONDITIONS
- SHEET 4 EXISTING SUBCATCHMENT AREAS
- SHEET 5 PROPOSED EQUIPMENT LAYOUT
- SHEETS 6-7 ACCESS ROAD LAYOUT & 345KV DUCT BANK LAYOUT
- SHEET 8 PROPOSED GRADING AND DRAINAGE
- SHEET 9 PROPOSED SUBCATCHMENT AREAS
- SHEET 10 TYPICAL DETAIL SHEET